# **Appendix F**

# **KDOW PAA Pilot Submittal**

April 24, 2017

Mr. Greg Goode, P.E. Env. Engineering Consultant Water Infrastructure Branch Division of Water 300 Sower Boulevard Frankfort, KY 40601

**Brandenburg Wastewater Facilities Plan** Re:

> **PAA Pilot Study Submittal GRW Project No. 4556**

Dear Mr. Goode:

Per our correspondence, the following is a notice of the City of Brandenburg's intention to complete a Peracetic Acid (PAA) Pilot at the existing Brandenburg Wastewater Treatment Plant (KPDES Permit No. KY0021474). The pilot will begin on approximately May 8, 2017 and proceed for six (6) months in order to get results during both wet and dry weather conditions.

Enclosed is a process flow schematic of the Brandenburg WWTP with the treatment train, PAA injection location, skid location, tote location, and sampling location for pilot. The pump during the pilot will be a Blue White M3 peristaltic pump, please see brochure attached. All hard piping is stainless steel and flexible Telfon tubing is used in locations where it's not hard piped. The pumps and piping will be located on a modified ChemPlus Simplex System skid, please see brochure attached. The skid will have a secondary containment for any leaks from the pumps. Lastly, please see attached for the contact times. Contact times for the design average (0.312 MGD), design peak (0.932 MGD), and average 2016 effluent (0.181 MGD) flows are 14.8, 10.6, and 16.9 minutes, respectively. See enclosed for calculations.

If you have any questions or comments, please do not hesitate to contact me at 502-489-8484, or jpavoni@grwinc.com

Sincerely,

Joseph V. Pavoni, P.E., LEED AP

Project Manager

Enclosure:

Process Flow Schematic of Brandenburg WWTP

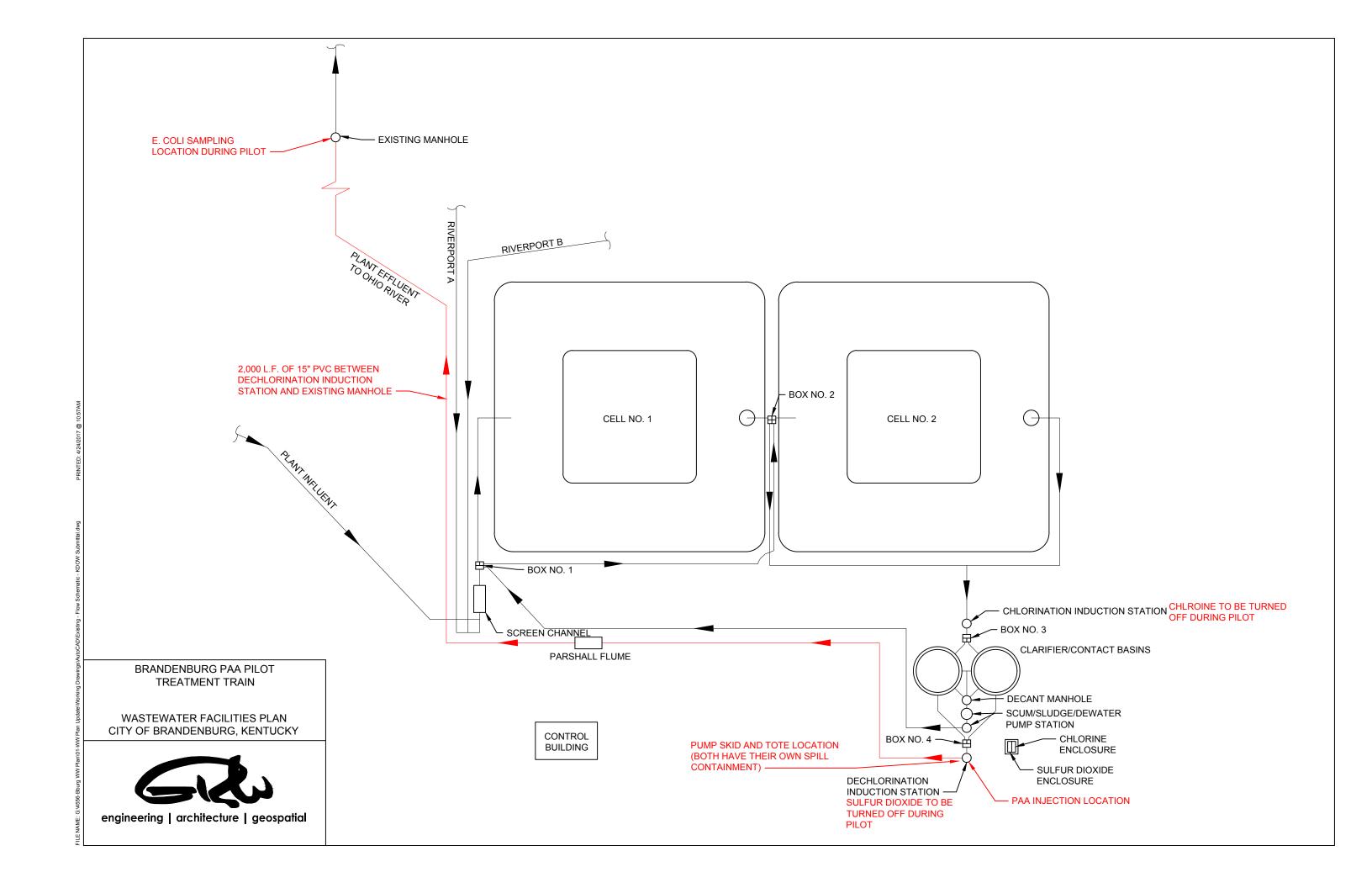
Blue White M3 Peristaltic Pump Brochure

ChemPlus Feed Systems Brochure

Contact Time Calculations

Cc: Mayor Ronnie Joyner, City of Brandenburg

T.J. Hughes, Brandenburg Public Works Director







# **Applications:**

- Municipal Water Treatment
- Municipal Wastewater Treatment
- Chemical Metering
- Chlorination
- Chloramination
- Fluoridation
- Polymer Injection
- Acid Injection
- Alum Injection
- PAC Injection
- Caustic Injection

# Features:

- Peristaltic pump design does not have valves that can clog requiring maintenance.
- Self priming even against maximum line pressure. By-pass valves are not required. Cannot vapor lock or lose prime.
- Output rates to: 158.5 GPH (600 LPH) and pressures to 125 PSI (8.6 Bar).
- 10,000:1 turndown ratio with high resolution motor speed adjustment.
- No maintenance brushless variable speed motor.
- Specially engineered tubing for long life at high pressures.
- Patented Tube Failure Detection (TFD) system. Senses tube failure by detecting chemical in the pump head. No false triggering from condensation or washdown.
- Control Inputs include: 4-20mA, 0-10Vdc, and Pulse inputs for remote external speed or batch control and 0-30 VDC / contact closure remote start/stop.
- Revolution count display with user programmable alarm set-point for tube maintenance.
- VGA Graphic multi-color backlit LCD displays remote/local control status, motor speed, output rate, input signal values, service and alarm status in three easy to see colors.
- Outputs include: Scalable 4-20mA or pulse, one 250V/6A relay and three 115V/1A contact closures assignable to monitor various pump functions including TFD, FVS, revolution counter, remote/local, forward/reverse, input signals, output signals, motor on, motor fault, operating mode setting, and others.
- Two CNC precision machined squeeze rollers and two alignment rollers provide factory calibrated optimum squeeze for unparalleled accuracy and extra long tube life.
- Heavy duty rotor single piece plastic rotor means no flexing and increased accuracy with no metal springs or hinges to corrode.
- Inject at maximum pressure in either direction (clockwise or counter clockwise).
- Compatible with Blue-White's output Flow Verification Sensor (FVS) system.

1 of 6 TDS #85000-108 rev. 11072016 ,

# **Engineering Specifications:**

# Maximum working pressure (excluding pump tubes):

125 psig (8.6 bar)

Note: see individual pump tube assembly maximum pressure ratings.

# Maximum Fluid temperature (excluding pump tubes):

3/8" OD x 1/4" ID tubing connections: 130°F (54 °C)

M/NPT connections: 185°F (85℃)

Note: see individual pump tube assembly maximum temperature ratings.

# **Ambient Operating Temperature**

14F to 115F (-10C to 46C)

# **Ambient Storage Temperature**

-40°F to 158°F° (-40°C To 70°C)°

## **Operating Voltage:**

M-3 MODELS: 96 to 264VAC-50/60Hz, 220 VA M-4 MODELS: 96 to 264VAC-50/60Hz, 350 VA

#### **Power Cord Options:**

115V60Hz = NEMA 5/15 (USA) 230V60Hz = NEMA 6/15 (USA) 220V50Hz = CEE 7/VII (EU)

240V50Hz = AS 3112 (Australia/New Zealand)

## **Enclosure:**

NEMA 4X (IP66), Polyester powder coated aluminum.

Maximum Overall Dimensions:

M-3 models: 8-1/8"W x 10-3/4"H x 15-1/4"D (20.6W x 27.3H x 38.9D cm) M-4 models: 12-1/8"W x 14-1/4"H x 18-5/8"D (30.8W x 36.1H x 47.3D cm)

# Approximate shipping wt:

M-3 models: 33 lb. (15.0 Kg) M-4 models: 58 lb. (26.3 Kg)

# Motor speed adjustment range

10,000:1 (0.001% - 100% motor speed)

# Motor speed adjustment resolution

0.1% increments > 10% motor speed 0.01% increments > 1% motor speed and < 10% 0.001% increments < 1% motor speed

#### **Maximum viscosity**

12,000 Centipoise

#### Maximum suction lift:

30 ft. Water, 0 psig (4.5 m, 0 bar)

# **Display**

3 color VGA backlit LCD, UV resistant.

# **Display resolution**

0.0 > 10% motor speed 0.00 > 1% motor speed and < 10% 0.000 < 1% motor speed

# Display languages

English, Spanish, French or German selectable.

#### Keypad

Eleven button positive action tactile switch keypad.

### Security

Programmable 4-digit password.

# **Materials of Construction:**

Wetted components:

### Pump Tube Assembly (Model Specific - 2 provided):

Tubing: . . . . . Norprene® or Norprene® Chemical or Tygothane®

Adapter fittings: .PVDF

# Recommended Ancillary Items Sold Separately:

#### Injection / Back-flow Check valve:

 Body & insert:
 PVDF

 Check Ball:
 Ceramic

 Spring:
 Hastelloy C-276

 Ball Seat O-ring:
 Viton\* (optional EP)

 Static Seal O-ring:
 Viton\* (optional EP)

Duckbill anti-scale valve: Santoprene®

# For "S" tubing type connections only:

(Available on M3 only)

Suction Tubing: . . . . . 3/8" OD x 1/4" ID x 10' Clear PVC

Discharge Tubing: . . . . 3/8" OD x 1/4" ID x 10' Polyethylene (LLDPE)

Suction Strainer: . . . . Polypropylene

# For "B" barb tubing and "M" M/NPT connections only:

(Available on M-4 and M-3 Flex-A-Prene® only)

**Suction Strainer:** 

Ball Seat O-ring: . . . . . . . Viton® (optional EP)

# For "C" Tri-clamp and "Q" Quick Disconnect connections\* only:

(Available on M-3 Flex-A-Prene® only) **Suction Strainer:** . . . . . Polypropylene

\*Quick Disconnect Valves sold separately

# Non-Wetted components:

# **Enclosure:**

413 Aluminum (Polyester powder coated)

#### Pump Head:

Valox® (PBT) thermoplastic

#### **Pump Head Cover:**

Polycarbonate for added strength and chemical resistance. Permanently lubricated sealed motor shaft support ball bearing.

## **Cover Screws:**

Stainless Steel

# Roller Assembly:

Rotor:.....Valox \* (PBT)
Rollers:....Nylon
Roller Bearings:...SS Ball Bearings

Koller Bearings.......33 Daii Bearin

# Motor Shaft:

Chrome plated steel

# **TFD System Sensor pins:**

Hastelloy C-276

# Power Cord:

3 conductor, SJTW-A Water-resistant

### **Tube Installation Tool:**

Glass Filled Nylon

# **Mounting Brackets and Hardware:**

316 Stainless Steel

# **Output Specifications:**

	Output Rar	ıge	Max Speed	Max Pressure	Max Temperature	M-3	Model Num	bers
Norpren Listed unde	SIP							
GPH	LPH	ML/MIN	RPM	PSI (bar)	F (C)	115V AC	230V AC	220V AC
.0002 - 2.10	.0007 - 7.92	.0132 - 132	125	125 (8.6)	185 (85)	M-324-*ND	M-325-*ND	M-326-*ND
.0025 - 25.3	.0096 - 96.0	.1596 - 1596	125	125 (8.6)	185 (85)	M-324-*NJ	M-325-*NJ	M-326-*NJ
.0033 - 33.3	.0126 - 126	.2100 - 2100	125	125 (8.6)	185 (85)	M-324-*NK	M-325-*NK	M-326-*NK
.0033 - 33.3	.0126 - 126	.2100 - 2100	125	30 (2.1)	185 (85)	M-324-*NKL	M-325-*NKL	M-326-*NKL
Flex-A-F	Prene® M	-3 Tube P	umps					
				food   Excel	lent chemical resi	stance   Extra	a long life at lo	w pressures
GPH	LPH	ML/MIN	RPM	PSI (bar)	F (C)	115V AC	230V AC	220V AC
.0005 - 4.8	.0018 - 18.0	.03 - 300	125	110 (7.6)	185 (85)	M-324-*NEE	M-325-*NEE	M-326-*NEE
.0019 - 19.0	.0072 - 72.0	.12 - 1200	125	110 (7.6)	185 (85)	M-324-*NGG	M-325-*NGG	M-326-*NGG
Norpren	ıe® Chem	ical M-3	<b>Tube P</b>	umps				
					rior chemical resi	stance		
GPH	LPH	ML/MIN	RPM	PSI (bar)	F (C)	115V AC	230V AC	220V AC
.0014 - 14.5	.0055 - 55.1	.0920 - 920	125	50 (3.4)	130 (54)	M-324-*TH	M-325-*TH	M-326-*TH
.0028 - 28.5	.0108 - 108	.1800 - 1800	125	50 (3.4)	130 (54)	M-324-*TK	M-325-*TK	M-326-*TK
Tygotha	ne® M-3	Tube Pun	nns					
		d   Resistant		eases and fue	els			
GPH	LPH	ML/MIN	RPM	PSI (bar)	F (C)	115V AC	230V AC	220V AC
.0004 - 4.60	.0017 - 17.4	.0290 - 290	125	65 (4.5)	130 (54)	M-324-*GE	M-325-*GE	M-326-*GE
.0010 - 10.1	.0038 - 38.4	.0637 - 637	125	65 (4.5)	130 (54)	M-324-*GG	M-325-*GG	M-326-*GG
.0024 - 24.9	.0094 - 94.2	.1570 - 1570	125	65 (4.5)	130 (54)	M-324-*GH	M-325-*GH	M-326-*GH
.0028 - 28.5	.0108 - 108	.1800 - 1800	125	65 (4.5)	130 (54)	M-324-*GK	M-325-*GK	M-326-*GK

	Output Rar	ıge	Max Speed	Max Pressure	Max Temperature	M-4	Model Nur	nbers
		ube Pum		food   Exce	llent chemical re	esistance   CIP	SIP	
GPH	LPH	ML/MIN	RPM	PSI (bar)	F (C)	115V AC	230V AC	220V AC
.0028 - 28.5	.0108 - 108	.180 - 1800	125	125 (8.6)	185 (85)	M-424-*NH	M-425-*NH	M-426-*NH
.0044 - 44.4	.0168 - 168	.280 - 2800	125	100 (6.9)	185 (85)	M-424-*NJ	M-425-*NJ	M-426-*NJ
.0050 - 50.7	.0192 - 192	.320 - 3200	125	80 (5.5)	185 (85)	M-424-*NK	M-425-*NK	M-426-*NK
.0054 - 54.0	.0204 - 204	.340 - 3400	125	100 (6.9)	185 (85)	M-424-*NHH	M-425-*NHH	M-426-*NHH
.010 - 100.0	.0378 - 378	.630 - 6300	125	50 (3.4)	185 (85)	M-424-*NL	M-425-*NL	M-426-*NL
.015 - 158.5	.0600 - 600	1.00 - 10000	125	30 (2.0)	185 (85)	M-424-*NP	M-425-*NP	M-426-*NP
Norpren	e <sup>®</sup> M-4 L	ow Press	ure Tul	be Pump	S			
Listed under	r NSF Std. 61	Meets FDA	criteria for	food   Exce	llent chemical re	esistance   Extr	a long life at	low pressures
GPH	LPH	ML/MIN	RPM	PSI (bar)	F (C)	115V AC	230V AC	220V AC
.0050 - 50.7	.0192 - 192	.320 - 3200	125	30 (2.1)	185 (85)	M-424-*NKL	M-425-*NKL	M-426-*NKL
.011 - 111.0	.0420 - 420	.700 - 7000	125	30 (2.1)	185 (85)	M-424-*NKKL	M-425-*NKKL	M-426-*NKKL
Norpren	e <sup>®</sup> Chem	ical M-4	Tube P	umps				
					erior chemical r	esistance		
GPH	LPH	ML/MIN	RPM	PSI (bar)	F (C)	115V AC	230V AC	220V AC
.0020 - 20.6	.0078 - 78.0	.130 - 1300	125	30 (2.1)	130 (54)	M-424-*TH	M-425-*TH	M-426-*TH
.0042 - 42.8	.0162 - 162	.270 - 2700	125	30 (2.1)	130 (54)	M-424-*TK	M-425-*TK	M-426-*TK
.0050 - 50.7	.0192 - 192	.320 - 3200	125	30 (2.1)	130 (54)	M-424-*THH	M-425-*THH	M-426-*THH
Tygotha	no® M 1	Tube Pun	nnc					
		od   Resistant		pases and fu	ale			
GPH	LPH	ML/MIN	RPM	PSI (bar)	F (C)	115V AC	230V AC	220V AC
.0039 - 39.6	.0150 - 150	.250 - 2500	125	65 (4.5)	130 (54)	M-424-*GH	M-425-*GH	M-426-*GH
				( )	\ /			
.0055 - 55.5	.0210 - 210	.350 - 3500	125	65 (4.5)	130 (54)	M-424-*GK	M-425-*GK	M-426-*GK

<sup>\*</sup> Inlet/outlet connection types available: **S** = 3/8" OD x 1/4" ID flexible tubing with compression type connections (M-3 models only), **M** = 1/2" male NPT, **B** = 1/2" ID tubing barb type connections (M-3 Flex-A-Prene® and M-4 models only), **C** = 3/4" tri-clamp connections (M-3 Flex-A-Prene® models only), **Q** = Quick Disconnect (M-3 Flex-A-Prene® models only) (Valves sold separately) Note: output volumes based on testing with water at 72 degrees F, 5 foot suction lift, atmospheric conditions at sea level.

**3 of 6** TDS #85000-108 rev. 11072016

# **Chemical Resistance of Tubing:**

# Flex-A-Prene® and Norprene® Tubing

Meets FDA criteria for food | Excellent chemical resistance

Aluminum Sulfate (Alum) Ammonium chloride Ammonium hydroxide Ammonium Sulfate (LAS) Benzyl alcohol

Brine solutions Calcium Hydroxide 10% (Lime Slurry) Calcium hypochlorite 20%

Citric Acid 50% Ethylene alvcol Ferric chloride Ferric nitrate Ferric sulfate Ferrous chloride - 43% in water Ferrous sulfate

Fluosilicic Acid (up to 25%) Formic acid Glucose

Hydrochloric acid 33% Hydrocvanic acid Hydrogen peroxide Hypochlorous acid lodine Magnesium chloride Magnesium sulfate

Phosphoric acid Plating solutions Polyaluminum Chloride (PAC)

Potassium hydroxide Potassium permanganate 40% Propylene glycol Sodium hydroxide 50% Sodium Bisulfite Sodium Chlorite 12% Sodium Hypochlorite 12.5% Sodium sulfide Sulfuric acid up to 50%

Tannic acid

## Chemical Tubing - Ultra smooth plasticizer-free bore (inner liner) Norprene<sup>6</sup>

Meets FDA criteria for food | Superior chemical resistance

Fluoboric Acid (up to 48%) Fluosilicic Acid (up to 25%) Hydrofluoric Acid (up to 48%) Nitric Acid (up to 71%) Phosphoric Acid (up to 85%) Potassium Hypochlorite (up to 70%) Sodium Phosphate (up to 30%) Sulfuric Acid (up to 98%)

Salts Ketones Alcohols Isobutyl Alcohol

Ink and solvent production Battery acid filling

Specialty chemical production / processing Sensitive fluid transfer

# Tygothane<sup>®</sup> **Tubing**

Meets FDA criteria for food | Resistant to oils, greases and fuels

Cyclohexane Diesel Fuel Fatty acids Gasoline Heptane Hexane

Kerosene Lard Mineral spirits Soap solutions Turpentine

ASTM reference No.1,2,3 Castor Coconut

Oils: Linseed Lubricating Mineral

Norprene is a registered trademark of Saint-Gobain. Tygothane is a registered trademark of Saint-Gobain

# **Pump Tube Assembly Output Specifications:**

Model M-3 - Available Pump Tubes and Output Ranges									
Tube Material	Tube Size	Max Output Pressure	Max Temp	Required Roller Part Number		Output Range			
Material	Code	PSI (bar)	F (C)		GPH	LPH	ML/Min		
Norprene Flex-A-Prene Flex-A-Prene	ND NEE NGG	125 (8.6) 110 (7.6) 110 (7.6)	185 (85) 185 (85) 185 (85)	A3-SND-R A3-SNEE-R A3-SNGG-R	.0002 - 2.10 .0005 - 4.8 .0019 - 19.0	.0007 - 7.92 .0018 - 18.0 .0072 - 72.0	.0132 - 132 .03 - 300 .12 - 1200		
Norprene Norprene	NJ NK	125 (8.6) 125 (8.6)	185 (85) 185 (85)	A3-SNH-R A3-SNH-R	.0025 - 25.3 .0033 - 33.3	.0096 - 96.0 .0126 - 126	.1596 - 1596 .2100 - 2100		
Norprene Norprene Chemical	NKL TH	30 (2.1) 50 (3.4)	185 (85) 130 (54) 130 (54)	A3-SNH-R A3-STH-R A3-STH-R	.0033 - 33.3 .0014 - 14.5 .0028 - 28.5	.0126 - 126 .0055 - 55.1	.2100 - 2100 .0920 - 920		
Norprene Chemical Tygothane Tygothane	TK GE GG	50 (3.4) 65 (4.5) 65 (4.5)	130 (54) 130 (54) 130 (54)	A3-SGE-R A3-SGE-R	.0028 - 28.5 .0004 - 4.60 .0010 - 10.1	.0108 - 108 .0017 - 17.4 .0038 - 38.4	.1800 - 1800 .0290 - 290 .0637 - 637		
Tygothane Tygothane	GH GK	65 (4.5) 65 (4.5)	130 (54) 130 (54)	A3-SGE-R A3-SGE-R	.0024 - 24.9 .0028 - 28.5	.0094 - 94.2 .0108 - 108	.1570 - 1570 .1800 - 1800		

Model M-4 - Available Pump Tubes and Output Ranges									
Tube Material	Tube Size	Max Output Pressure	Max Temp	Required Roller Part Number		Output Range			
Material	Code	PSI (bar)	F (C)		GPH	LPH	ML/Min		
Norprene	NH	125 (8.6)	185 (85)	A4-MNH-R	.0028 - 28.5	.0108 - 108	.1800 - 1800		
Norprene	NJ	100 (6.9)	185 (85)	A4-MNH-R	.0044 - 44.4	.0168 - 168	.2800 - 2800		
Norprene	NK	80 (5.5)	185 (85)	A4-MNH-R	.0050 - 50.7	.0192 - 192	.3200 - 3200		
Norprene	NHH	100 (6.9)	185 (85)	A4-MNH-R	.0054 - 54.0	.0204 - 204	.3400 - 3400		
Norprene	NL	50 (3.4)	185 (85)	A4-MNL-R	.0100 - 100.0	.0378 - 378	.6300 - 6300		
Norprene	NP	30 (2.1)	185 (85)	A4-MNL-R	.0158 - 158.5	.0600 - 600	1.00 - 10000		
Norprene	NKL	30 (2.1)	185 (85)	A4-MNH-R	.0050 - 50.7	.0192 - 192	.3200 - 3200		
Norprene	NKKL	30 (2.1)	185 (85)	A4-MNH-R	.0111 - 111.0	.0420 - 420	.7000 - 7000		
Norprene Chemical	TH	30 (2.1)	130 (54)	A4-MTH-R	.0020 - 20.6	.0078 - 78.0	.1300 - 1300		
Norprene Chemical	TK	30 (2.1)	130 (54)	A4-MTH-R	.0042 - 42.8	.0162 - 162	.2700 - 2700		
Norprene Chemical	THH	30 (2.1)	130 (54)	A4-MTH-R	.0050 - 50.7	.0192 - 192	.3200 - 3200		
Tygothane	GH	65 (4.5)	130 (54)	A4-MGH-R	.0039 - 39.6	.0150 - 150	.2500 - 2500		
Tygothane	GK	65 (4.5)	130 (54)	A4-MGH-R	.0055 - 55.5	.0210 - 210	.3500 - 3500		
Tygothane	GKK	65 (4.5)	130 (54)	A4-MGH-R	.0100 - 100.0	.0378 - 378	.6300 - 6300		

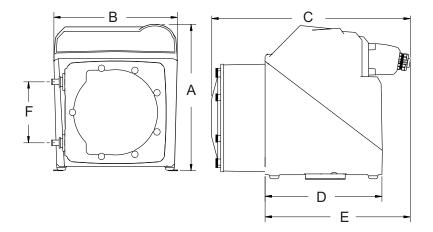
TDS #85000-108 rev. 11072016



# FLEX-PRO® ProSeries-M® Peristaltic Metering Pump

# Engineering and Technical Data

# **Dimensions:**



	M-3 S	Series	M-4 Series		
Dim	Inches	cm	Inches	cm	
Α	10-3/4"	27.3	14-1/4"	36.1	
В	8-1/8"	20.6	12-1/8"	30.8	
С	15-1/4"	38.9	18-5/8"	47.3	
D	10"	25.4	11"	27.9	
Е	12-1/4"	31.0	13-5/8"	34.6	
F	4-1/4"	10.7	6"	15.2	

# **Model Number Matrix:**

M	Flex	เ-Pro	ProS	eries-	M <sup>®</sup> Model N	lum	ber								
	Max	cimur	n Out	put Ra	nge										
	3	Flex-	lex-Pro M-3 maximum output 33.3 GPH(126 LPH)												
	4	Flex-	-Pro M	ro M-4 maximum output 158.5 GPH (600 LPH)											
		Max	cimun	mum Motor Speed											
		2	125	125 RPM (maximum rotor rotation speed)											
			Pow	Power Cord (operating voltage requirement 96VAC to 264VAC)											
			4	115V /	60Hz, powe	r co	rd NEMA 5/15 plug (US)								
			5	230V	60Hz, powe	r cc	ord NEMA 6/15 plug (US)								
			6	220V	/ 50HZ, powe	er co	ord CEE 7/VII plug (EU)								
			8	240V	/ 50HZ, powe	er co	ord AS 3112 plug (Australia/New zealand	d)							
			X	No Po	wer Cord										
				Inl	et/Outlet Co	onn	ection Size, Connection Type, Conn	ection	Material						
				S	3/8" OD x 1	/4" I	D Tube Compression Fitting, Natural P\	DF (K	ynar), available for M-3 models only						
				М	1/2" Male NF	T F	itting, Natural PVDF (Kynar), available f	or M-3	and M-4 models						
				В	½" ID Tubin	g Ba	arb Fitting, Natural PVDF (Kynar), availa	ble for	M-4 models only						
				С	1/2" - 3/4" T	ri-cl	amp connections, Natural PVDF (Kynar	), availa	able for M-3 Flex-A-Prene® models only						
				Q Quick Disconnect, Natural PVDF (Kynar), available on M-3 models only, Flex-A-Prene® only, (Valves sold separately)											
					Pump Tub	е М	aterial, Pump Tube Size								
					ND	No	orprene® .075 ID, M-3 models only	THH	Norprene®Chemical .250 ID (Dual Tube), M-4 models only						
					NH	No	orprene®.250 ID - M-4 models only	TK	Norprene®Chemical .375 ID						
					NHL	No	orprene®.250 ID (65 PSI max pressure - longer life) M-4 only	TKK	Norprene®Chemical .375 ID (Dual Tube), M-4 models only						
					NHH	-	orprene®.250 ID (Dual Tube), M-4 models only	GE	Tygothane®.125 ID, M-3 models only						
					NJ	_	orprene®.312 ID	GG	Tygothane®.187 ID, M-3 models only						
					NK	-	orprene <sup>®</sup> .375 ID	GH	Tygothane®.250 ID						
					NKL	_	orprene®.375 ID (30 PSI max pressure - longer life)	_	Tygothane®.250 ID (Dual Tube), M-4 models only						
					NKK	_ No	orprene 375 ID (30 PSI max - Dual Tube), M-4 models only	GK	Tygothane®.375 ID						
				NL Norprene .500 ID, M-4 models only GKK Tygothane .375 ID (Dual Tube), M-4 models only											
				NP Norprene®.750 ID, M-4 models only NEE Flex-A-Prene®.093 ID (Dual Tube), M-3 models only											
			TH Norprene®Chemical .250 ID NGG Flex-A-Prene®.187 ID (Dual Tube), M-3 models only												
				R Right facing pump head, input / output (Left facing fluid input / output is standard)											
						D	Down facing pump head, input / output (	Left fac	sing fluid input / output is standard)						
	<u> </u>	<u> </u>	<u> </u>	<b>V</b>	<b>+</b>	<b>₩</b>									
М	- 4	2	4	- M	NH		Sample Model Number								

# **Features list:**

#### **Features**

TFD (Tube Failure Detection) System Alarm

FVS (Flow Verification System) Alarm - Requires Micro-Flo Sensor (sold separately)

10,000:1 turndown reversible motor.

Three position pump head rotation

Tube Life revolution counter with user programmable alarm set-point.

Set maximum motor RPM limit.

Power interruption re-start options.

Four output contacts can be triggered by the status of the TFD system, FVS system, general alarm, rotor direction, motor run/stop, revolution count, motor fail, remote/local setting, input signal failure, output signal failure, and currently active operating mode.

Output: One, 6 amp alarm relay

Output: Three, dry contact or maximum 30VDC/115VAC 1 amp contact closures

Output: Programmable 4-20mA signal or high speed pulse, proportional to pump output

Input: One, contact closure (remote start / stop)

Input: Remote speed control via 4-20mA, 0-10VDC, high speed digital pulse, contact closure pulse

Remote/Local control settings

Password protection

Display: Motor speed, Tube life revolutions, Tube Failure Detection (TFD) system and Flow Verification System (FVS) alarm status

Display: Output in ml/min, oz/min, L/hr, Gal/hr, Gal/day, RPM, and input signal values

Automated PPM chemical dosing system

## **Available Operating Modes:**

Manual (local): speed adjustment

Remote input: 4-20mA

Remote input: 0-10 VDC

Remote input: high speed frequency (pulse) input

Remote input: pulse triggered batch dispensing

Remote input: proportional PPM (parts per million) dosing with high speed frequency (pulse) input

Manual (local): batch dispensing

Manual (local): repeating cycle timer

Manual (local): fixed speed PPM (parts per million) dosing

Factory Authorized Representative:







5300 Business Drive, Huntington Beach, CA 92649 **Tel:** 714-893-8529 **Fax:** 714-894-9492 www.ProSeries-M.com Email: sales@blue-white.com

Patents: 4,496,295, 7,001,153, 8,418,364 and other patents pending

<sup>\*</sup> Requires Micro-Flo Sensor sold separately





# BPE BURT PROCESS

100 Overlook Drive Hamden CT 06514 Tel: (203) 287-1985 Fax: (203) 288-7354 Email: customerservice@burtprocess.com

www.burtprocess.com



Founded in 1970, Burt Process Equipment is a worldwide leader in design, manufacturing and distribution of fluid handling equipment, systems and services. BPE products and services provide proven solutions for a multitude industries. Burt Process Equipment is a one-stop, dependable source specializing in prompt delivery from the most comprehensive inventory of fluid handling equipment. Our services include complete engineered system design, turnkey system fabrication, field service and on-site technical and sales support.

# **Applications**

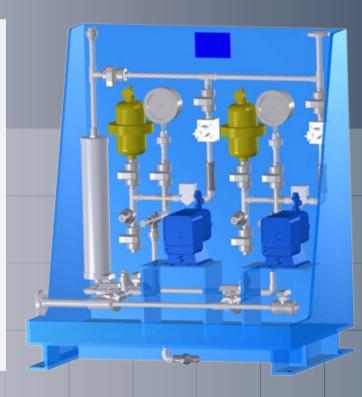
Municipal Water: Disinfection, Sodium Hypochlorite, pH Adjustment, Fluoride, Coagulation & Flocculation

Municipal Wastewater: Fume Scrubbers, pH Adjust, Odor Control, Residual Disinfectant Management

Food & Beverage: Clean-In-Place, Clean-off-Line, Sterilizer Water Treatment

Institutional: Cooling Tower Waste Treatment, Boiler Water Treatment, Closed Loop Systems

Other Applications: in Swimming Pools & Water Parks, Mining & Agriculture, Utilities & Dairy Industry, Pulp & Paper, Oil & Gas



# **ChemPlus Chemical Metering Systems**

The Burt Process **ChemPlus** has revolutionized the concept of chemical dosing systems by providing a fully engineered, pre-packaged, corrosion resistant system that provides complete flexibility in customization. ChemPlus gives engineers and systems designers the ultimate in flexibility with the latest advances and widest range of proven technologies. Burt Process draws on over 45 years of experience in specifying and manufacturing chemical processing equipment to provide a feed system design that meets the most rigorous chemical metering demands.

# CHEMPLUS Chemical Feed Systems

ChemPLUS pre-engineered systems offer a wide range of component options, pump selection and piping arrangements needed to customize chemical metering system solutions.

- Wide range of pump technologies include:
- Solenoid or Motor Driven Diaphragm Pumps
- Gear, Peristaltic, Progressing Cavity & more
- Rugged Acid, Caustic & Solvent Resistant Construction
- Complete System Fabrication, Assembly & Test
- Skid & Containment Basin constructed of 1/2" HDPE, Wall or Floor mountable
- Simplex, Duplex, Triplex or Custom Designs
- Piping materials in PP, PVC, CPVC, PVDF, or SS
- Standard & Custom Control Options Available



**ChemPlus Simplex System** 



**ChemPlus Duplex System** 



**ChemPlus Triplex System** 

# **System Features & Options**



# **Standard System Configurations**

Simplex w/selectable components

Duplex arrangements include:

- Redundant suction & discharge
- Redundant discharge only
- Lead/Backup, single pipe system

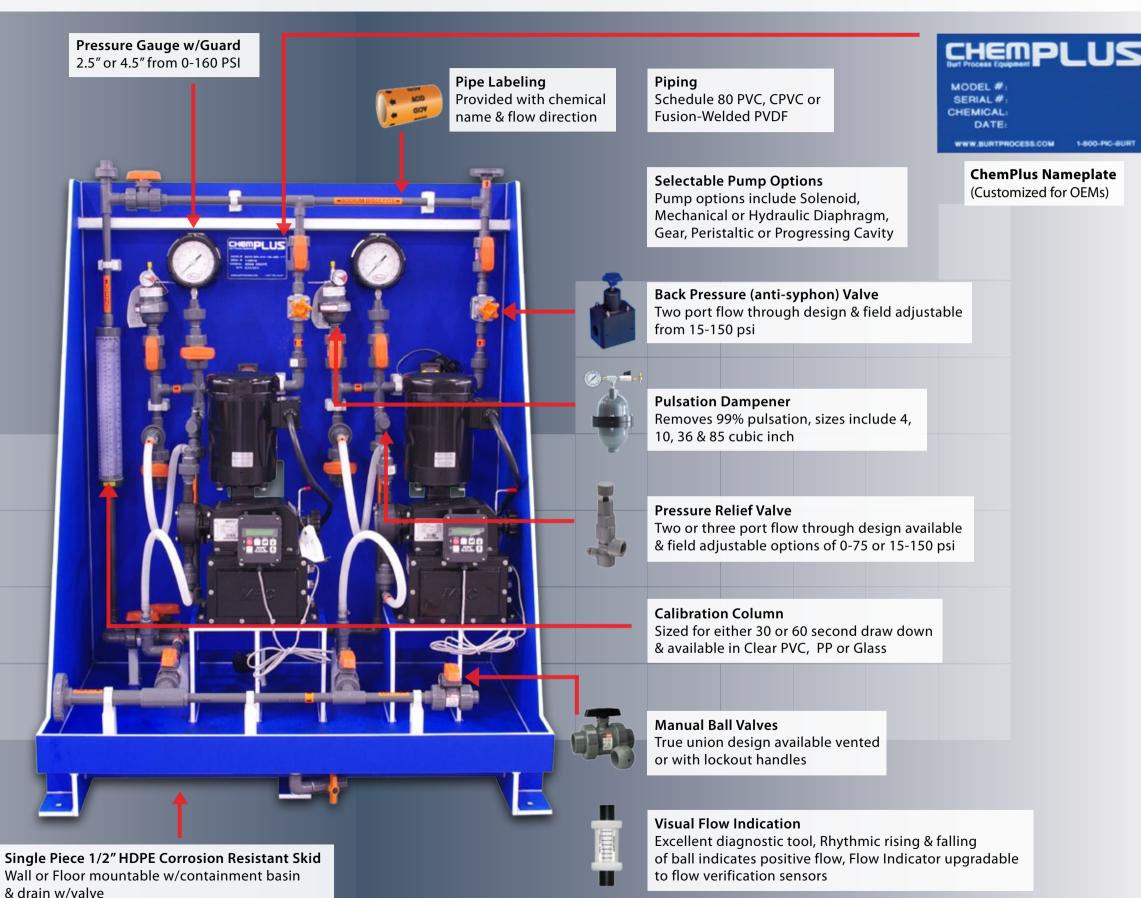
Triplex arrangements include:

- Redundant suction & discharge
- Redundant discharge only

# **Documented Factory Testing**

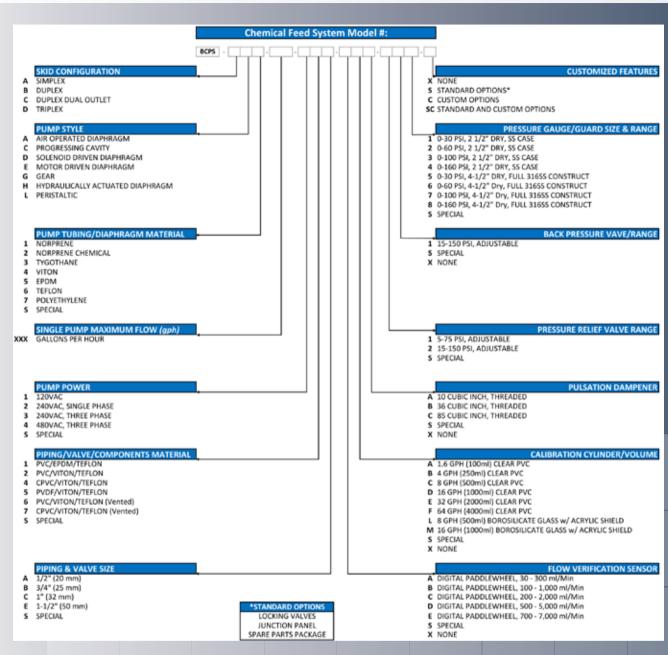
- √ 120-Minute Hydrostatic Pressure Test
- √ Pressure Relief Valve Preset & Tested
- ✓ Back Pressure Valve Preset & Tested
- ✓ Pulsation Dampener Charged & Tested
- ✓ Completed Pre-Shipment Check List
- ✓ Documented Inspection Report





www.burtprocess.com





# **System Options**

- Flow or Pressure Verification Sensors
- Conduit Box for Power & Control Signals
- Splash Guard Covers
- ChemPlus Nameplate may be customized
- HDPE or FRP Outdoor Shelters
- Elevated Support Legs or Stand
- Custom Controls & Instrumentation



**Simplex Unit** 

# CHEMPIUS Custom ChemPlus Systems

# **Custom Systems**

Choose from a variety of standard ChemPlus Systems, or the Burt Process team of highly qualified design engineers can develop a system to meet your specific application needs.

All systems are assembled at our facility on fabricated skids with electrical connections and controls as required.









# **Control Panels**

Burt Process Equipment provides complete industrial control panel design and UL 508A fabrication services as part of our product offering. Burt Process control panels provide the highest quality in workmanship coupled with optimum control in standard and custom enclosures. Interface modules available for Modbus, GENIbus, BACnet, Profibus, PROFINET, GSM/GPRS (wireless communication). All panels are UL 508A listed.

# **Control Panel Options**

- UL Type 4X FRP or stainless steel enclosures for corrosive environments
- Microprocessor based controllers
- Explosion proof rated panels for hazardous environments
- PLC (Programmable Logic Controller) and HMI (Human Machine Interface)
- PC Interface and data acquisition systems



# Burt Process also manufactures these reliable, pre-engineered systems



**PolymerPlus** 







www.burtprocess.com

**RainEx pHPlus TransferPlus** 

# **Duplex Unit w/ Splash Guard**

www.burtprocess.com





# **Burt Process is a proud Distributor of:**

# **Pumps**

AMT

Ansimag

ARO

Barnant

Blacoh

Blue-White

Ebara

Eastern

ECO

Eclipse

Gorman Rupp

Finish Thompson

Fluid-o-Tech

Flux

Grundfos

Hayward

Ismatec

Isochem

Liberty

Lutz

LMI

March

Masterflex

Magnatex

Micropump

# Milton Roy

Netzsch

Neptune

Oberdorfer

Price

Pulsafeeder

Pulsatron

Sundyne

Webster

# Instrumentation & Controls

Blue White

Dwyer

Flowline

GF

Signet

# Valves & Fittings

Asahi

GF Valves

Hayward

Plastomatic

Simtech

# Hose & Fittings

Barnant

Colder

Flex Tubing

Masterflex

Nalgene

# Tanks & Mixers

Chemtainer

Cleveland Eastern

Fusion Fluid

Nalgene

Norwesco

Peabody

# Motors & Drives

ABB

AC Tech

Baldor

Emotron

Siemens

Weg



# Corporate Headquarters

100 Overlook Drive Hamden, CT 06514 T: (203) 287-1985 F: (203) 288-7354

# **Sales Locations**

# California

T: (877) 742-2878 F: (877) 742-2231

# **New England**

T: (978) 649-9660 F: (978) 649-7430

# **New York**

T: (518) 477-5005 F: (518) 477-5008

# Connecticut

T: (203) 287-1985 F: (203) 288-7354

# **Mid-Atlantic**

T: (717) 792-6950 F: (717) 792-6953

BURT PROCESS

www.chemfeedonline.com www.polymerfeed.com

SUBMITTAL CALCULATIONS								
Project:	Brandenburg Facilities Plan							
Owner:	City of Brandenburg							
GRW Project No.:	4556							
Date:	April 24, 2017	Cal. By:	NWG					
Desc.:	Contact Time for PAA Pilot							

**Known Values:** 

Design Average Flow: 0.312 MGD Design Peak Flow: 0.932 MGD

Average Historic Effluent Flow (2016): 0.181 MGD

Pipe Diameter: 15-inch PVC

Approx. Length from Dechlorination Induction Station and Existing Manhole: 2,000 LF

Slope: 0.002 ft/ft (from Record Drawings)

n: 0.009

# Full Pipe Flow Calculations:

$$A = \pi (0.625)^2 = 1.227 ft^2$$

$$WP = 2\pi (0.625) = 3.927 ft$$

$$R = \frac{A}{WP} = \frac{1.227}{3.927} = 0.3125 ft$$

$$Q_{full} = \left(\frac{1.49}{n}\right) (A) \left(R^{2/3}\right) \left(\sqrt{S}\right) = \left(\frac{1.49}{0.009}\right) (1.227) \left(0.3125^{2/3}\right) \left(\sqrt{0.002}\right) = 4.18 ft^3/_{sec} \approx 2.7 \, \text{MGD}$$

$$V_{full} = \left(\frac{4.18}{\pi (0.625^2)}\right) = 3.41 \, ft/_{sec}$$

Average Influent Design Flow Contact Time:

$$\frac{\textit{Q}}{\textit{Q}_{full}} = 0.12 \rightarrow \frac{\textit{V}}{\textit{V}_{full}} \approx 0.66 \rightarrow \textit{V} = 2.25 \ \textit{ft/}_{\textit{Sec}} \ \text{(Using circular channel ratios graph)}$$
 
$$t_d = \frac{2000}{2.25} = 888.89 \ \textit{sec} = 14.8 \ \textit{min}.$$

Peak Influent Design Flow Contact Time:

$$\frac{Q}{Q_{full}}=0.35 
ightarrow \frac{V}{V_{full}} pprox 0.92 
ightarrow V=3.14 \ ^{ft}/_{sec}$$
 (Using circular channel ratios graph)  $t_d=\frac{2000}{3.14}=636.94 \ sec=10.6 \ min.$ 

Average Historic Effluent Flow Contact Time:

$$\frac{\textit{Q}}{\textit{Q}_{full}} = 0.07 \rightarrow \frac{\textit{V}}{\textit{V}_{full}} \approx 0.58 \rightarrow \textit{V} = 1.98 \ \textit{ft/}_{\textit{SeC}} \ \text{(Using circular channel ratios graph)}$$
 
$$t_d = \frac{2000}{1.98} = 1011.22 \ \textit{sec} = 16.9 \ \textit{min}.$$

# **Appendix G**

**Existing Rates and Charges** 

# City of Brandenburg

737 High Street P.O. Box 305

Brandenburg, KY 40108

(270)422-4981

Fax: (270)422-4983

Office Hours: Monday-Friday

8:00am-4:30pm

Afterhours Water/Sewer

Emergency:

(270)422-3774

Like us on



@ Brandenburg City Hall for announcements and updates!

\*Garbage Pick-up in the City Limits is on Tuesday and Friday weekly.

\*Recycle <u>in the City Limits</u> is on Tuesday only.

\*All water bills are due on the 15th of each month, penalties will be applied on the 16th, and shutoffs will be done on the 26th of each month.

\*Water meters are read monthly to determine usage, the sewer portion of your bill is based on the amount of water consumption per month.

# \*Inside City Limits\* Current Water Rates

First 2000 Gals./Month or lcss = \$11.68
next 3000 Gals./Month per 1000 = \$3.36
next 5000 Gals./Month per 1000 = \$3.11
next 5000 Gals./Month per 1000 = \$2.71
next 15,000 Gals./Month per 1000 = \$2.24
over 45,000 Gals./Month per 1000 = \$1.87

# Current Sewer Rates

First 2000 Gals./Month or less = \$15.11
next 3000 Gals./Month per 1000 = \$5.54
next 5000 Gals./Month per 1000 = \$5.33
next 20,000 Gals./Month per 1000 = \$5.05
next 15,000 Gals./Month per 1000 = \$4.64
over 45,000 Gals./Month per 1000 = \$4.34

# \*Outside City Limits\* Current Water Rates

First 2000 Gals./Month or less = \$18.01 next 3000 Gals./Month per 1000 = \$5.20 next 5000 Gals./Month per 1000 = \$4.77 next 20,000 Gals./Month per 1000 = \$4.20 next 15.000 Gals./Month per 1000 = \$3.47 over 45,000 Gals./Month per 1000 = \$2.89

# Current Sewer Rates

First 2000 Gals./Month or less = \$16.07
next 3000 Gals./Month per 1000 = \$5.83
next 5000 Gals./Month per 1000 = \$5.54
next 20.000 Gals./Month per 1000 = \$5.25
next 15.000 Gals./Month per 1000 = \$4.85
over 45.000 Gals./Month per 1000 = \$4.50

# **Appendix H**

# **Crosscutter Correspondence**

**U.S. Fish and Wildlife Service** 

**KY Department of Fish and Wildlife** 

**Kentucky Heritage Council** 

**U.S. Natural Resources Conservation Service** 

**U.S. Army Corps of Engineers** 

# City of Brandenburg

737 HIGH STREET POST OFFICE BOX 305 BRANDENBURG, KENTUCKY 40108 PHONE 270-422-4981 FAX 270-422-4983

MAYOR Ronnie Joyner

CITY COUNCIL
Bruce Fackler
Chris Hulsey
Bryan Claycomb
Patsy Lusk
Maggie Love
Scotty Applegate

CLERK/TREASURER Amy Haynes

Mr. Russell Neal
Environmental Control Supervisor
Wastewater Municipal Planning &
Capacity Development Section
Water Infrastructure Branch
Division of Water
300 Sower Boulevard
Frankfort, KY 40601

Re: City of Brandenburg Wastewater Facilities Plan

POLICE CHIEF Scotty Singleton

Cross-Cutter Review and Approval

proval

GRW Project No. 4556

PUBLIC WORKS DIRECTOR

Timothy J. Hughes, Jr.

Dear Mr. Neal:

October 27, 2017

GRW has explained your comments concerning the Cross-Cutter correspondence included in the Facilities Plan. The general request for review of proposed projects in the 20-year planning area have been submitted, and the responses stating that each project will need to be submitted when more detailed information is available.

We interpret these comments from the Cross-Cutters to mean that we should submit our projects when design is well underway, and decisions concerning processes and location of facilities are more concrete and detailed. We will certainly follow these guidelines to submit to each of the agencies when we are at an appropriate benchmark in the design phase.

Please feel free to contact me or Joe Pavoni at GRW if you have any other questions or comments.

Sincerely,

Ronnie Joyner

Mayor, City of Brandenburg

Cc: Joseph V. Pavoni, PE, LEED AP, GRW

Mr. Lee Andrews
Field Supervisor
U.S. Department of Interior
Fish and Wildlife Service
J.C. Watts Federal Building
330 West Broadway, Suite 265
Frankfort, KY 40601

Re: Cross-Cutter Correspondence Wastewater Facilities Plan Planning Period 2017-2037 City of Brandenburg, Kentucky GRW Project No. 4556

Dear Mr. Andrews:

On behalf of the City of Brandenburg, GRW is preparing a Wastewater Facilities plan pursuant to the regulations of the Kentucky Division of Water, as required by 401 KAR 5:006. The purpose of this plan is to develop recommendations for a series of projects that will be constructed to improve wastewater collection and treatment in the City's Planning Area of the Planning Period of 2017 to 2037. Exhibit 2-6 illustrates the Planning Area Boundary and planning phases. Exhibit 5-1 illustrates the existing wastewater collection system.

The Facilities Plan includes a series of projects that will be completed over the next 20 years in phases. Some of these projects will be built within the wastewater collection system or at the treatment plant site.

The collection system projects include new gravity sewers, new lift stations and force mains, constructed in easements or along existing road right-of-way in order to sewer existing neighborhoods (see Exhibit 1-4). These collection system projects are expected to be completed within the 3-10 and 11-20 year planning phases. The City may or may not choose to construct the collection system projects.

The treatment plant project (see Exhibit 6-3.1), expected to be completed by December 31, 2021, includes concrete treatment structures, buried pipes, rehabilitation of existing structures, and the abandonment and/or demolition of some existing plant components that are no longer required for treatment. The treatment plant project will be on the existing plant site that was disturbed by the original construction of the plant in 1993. The discharge of treated wastewater will remain at its existing location, which is on the Ohio River at mile point 643.3, segment 08217. The average daily discharge rate is projected to increase from its current flow rate of approximately 0.232 MGD (million gallons per day) to 0.268 MGD by 2037. The existing WWTP, however, is already rated to treat 0.312 MGD.

We would appreciate your advice of any concerns your office may have related to possible adverse effects of these projects as soon as possible. We need to incorporate your response in the Facilities Plan, and also address your concerns regarding any potential adverse impacts of these projects.

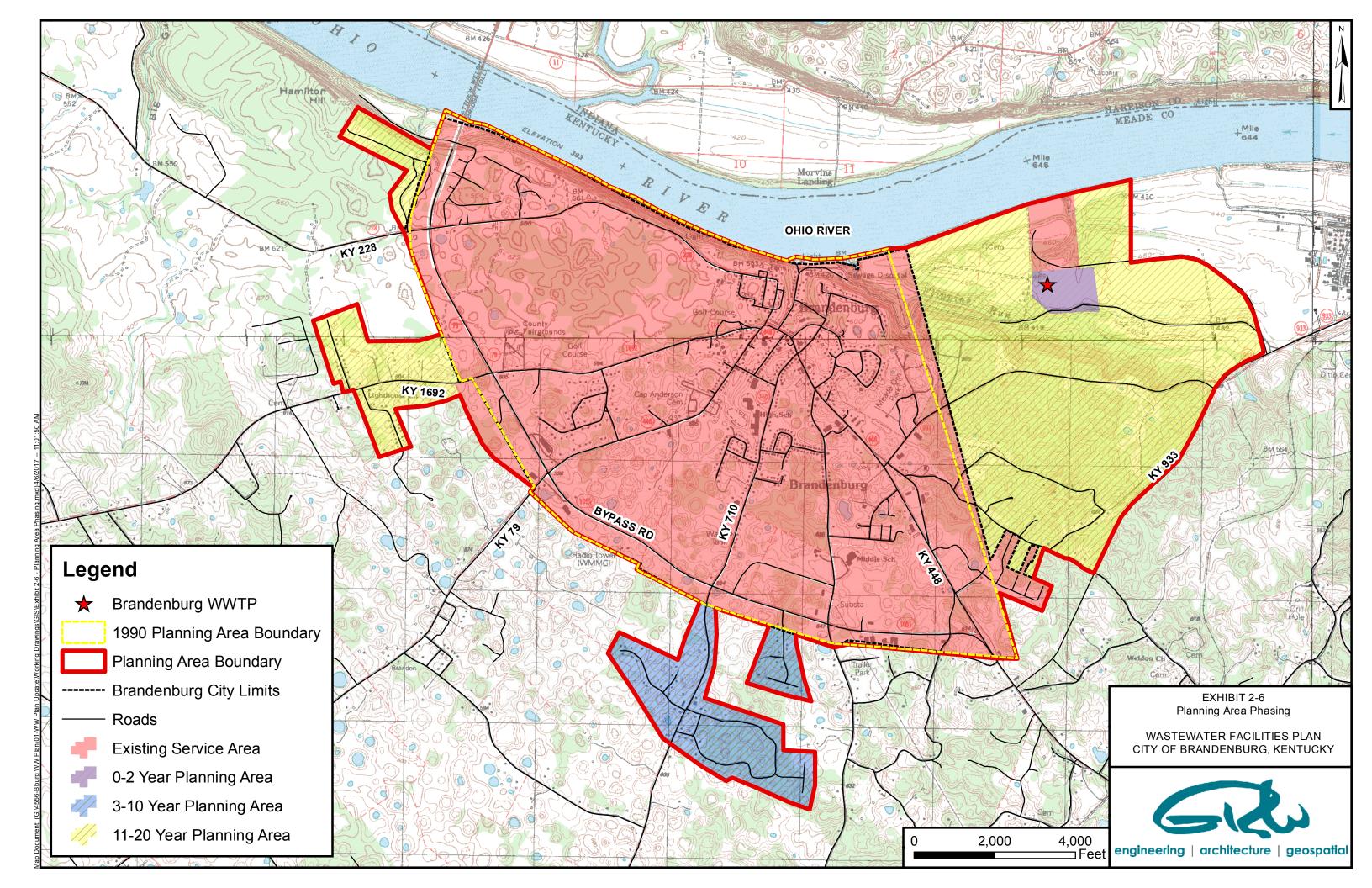
It is anticipated that these projects will be funded by a series of grants and loans, such as the USDA Rural Development Agency Grant and Loan Program and the Kentucky Infrastructure Agency State Revolving Fund Loan Program.

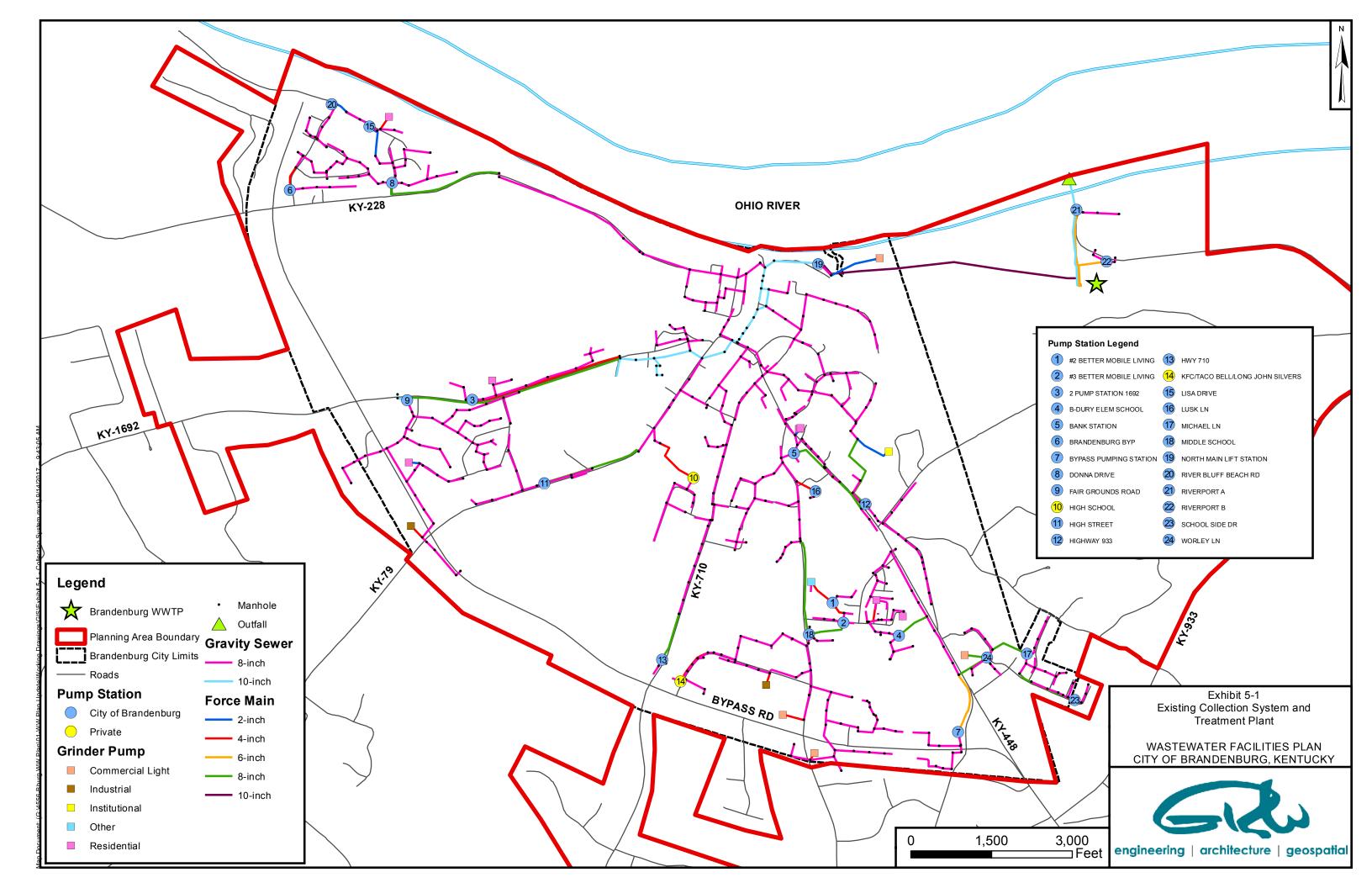
Please feel free to contact me if you have any questions or comments. I may be reached at 502-489-8484 or at ngunselman@grwinc.com

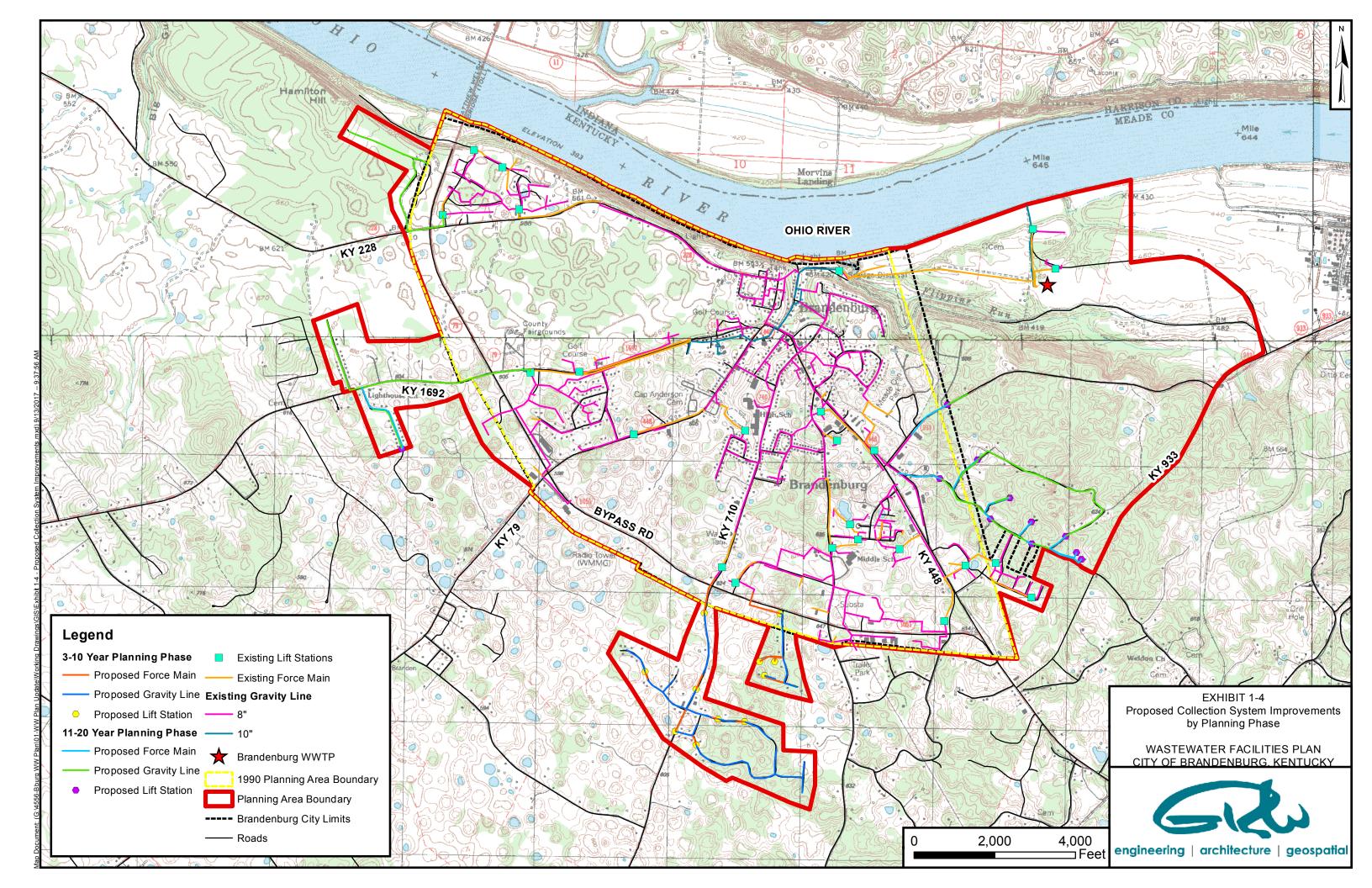
Sincerely,

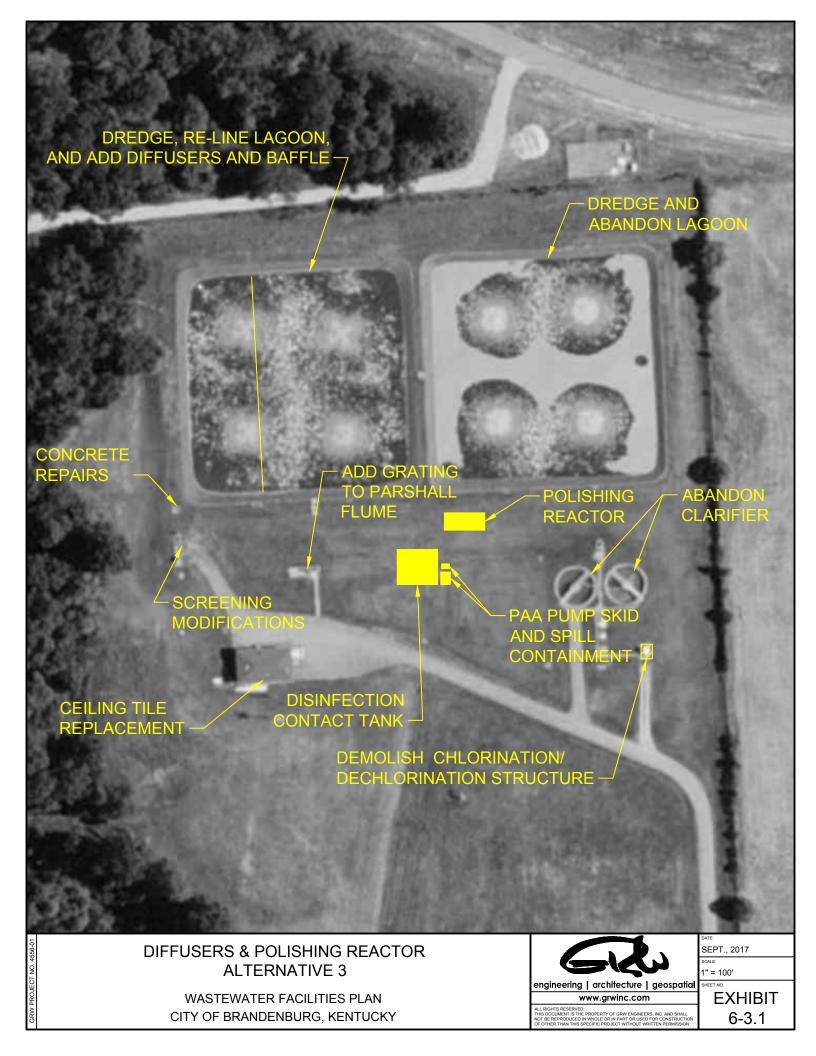
Nicholas Gunselman, P.E.

Project Engineer









Mr. Steve Beam Wildlife Director Kentucky Department of Fish and Wildlife Resources #1 Sportsman's Lane Frankfort, KY 40601 **Re:** Cross-Cutter Correspondence

Wastewater Facilities Plan Planning Period 2017-2037 City of Brandenburg, Kentucky

**GRW Project No. 4556** 

Dear Mr. Beam:

On behalf of the City of Brandenburg, GRW is preparing a Wastewater Facilities plan pursuant to the regulations of the Kentucky Division of Water, as required by 401 KAR 5:006. The purpose of this plan is to develop recommendations for a series of projects that will be constructed to improve wastewater collection and treatment in the City's Planning Area of the Planning Period of 2017 to 2037. Exhibit 2-6 illustrates the Planning Area Boundary and planning phases. Exhibit 5-1 illustrates the existing wastewater collection system.

The Facilities Plan includes a series of projects that will be completed over the next 20 years in phases. Some of these projects will be built within the wastewater collection system or at the treatment plant site.

The collection system projects include new gravity sewers, new lift stations and force mains, constructed in easements or along existing road right-of-way in order to sewer existing neighborhoods (see Exhibit 1-4). These collection system projects are expected to be completed within the 3-10 and 11-20 year planning phases. The City may or may not choose to construct the collection system projects.

The treatment plant project (see Exhibit 6-3.1), expected to be completed by December 31, 2021, includes concrete treatment structures, buried pipes, rehabilitation of existing structures, and the abandonment and/or demolition of some existing plant components that are no longer required for treatment. The treatment plant project will be on the existing plant site that was disturbed by the original construction of the plant in 1993. The discharge of treated wastewater will remain at its existing location, which is on the Ohio River at mile point 643.3, segment 08217. The average daily discharge rate is projected to increase from its current flow rate of approximately 0.232 MGD (million gallons per day) to 0.268 MGD by 2037. The existing WWTP, however, is already rated to treat 0.312 MGD.

We would appreciate your advice of any concerns your office may have related to possible adverse effects of these projects as soon as possible. We need to incorporate your response in the Facilities Plan, and also address your concerns regarding any potential adverse impacts of these projects.

It is anticipated that these projects will be funded by a series of grants and loans, such as the USDA Rural Development Agency Grant and Loan Program and the Kentucky Infrastructure Agency State Revolving Fund Loan Program.

Please feel free to contact me if you have any questions or comments. I may be reached at 502-489-8484 or at ngunselman@grwinc.com

Sincerely,

Nicholas Gunselman, P.E.

Project Engineer

Mr. Craig Potts Executive Director and State Preservation Officer Kentucky Heritage Council 410 High Street Frankfort, KY 40601 Cross-Cutter Correspondence Wastewater Facilities Plan Planning Period 2017-2037 City of Brandenburg, Kentucky GRW Project No. 4556

Dear Mr. Potts:

On behalf of the City of Brandenburg, GRW is preparing a Wastewater Facilities plan pursuant to the regulations of the Kentucky Division of Water, as required by 401 KAR 5:006. The purpose of this plan is to develop recommendations for a series of projects that will be constructed to improve wastewater collection and treatment in the City's Planning Area of the Planning Period of 2017 to 2037. Exhibit 2-6 illustrates the Planning Area Boundary and planning phases. Exhibit 5-1 illustrates the existing wastewater collection system.

The Facilities Plan includes a series of projects that will be completed over the next 20 years in phases. Some of these projects will be built within the wastewater collection system or at the treatment plant site.

The collection system projects include new gravity sewers, new lift stations and force mains, constructed in easements or along existing road right-of-way in order to sewer existing neighborhoods (see Exhibit 1-4). These collection system projects are expected to be completed within the 3-10 and 11-20 year planning phases. The City may or may not choose to construct the collection system projects.

The treatment plant project (see Exhibit 6-3.1), expected to be completed by December 31, 2021, includes concrete treatment structures, buried pipes, rehabilitation of existing structures, and the abandonment and/or demolition of some existing plant components that are no longer required for treatment. The treatment plant project will be on the existing plant site that was disturbed by the original construction of the plant in 1993. The discharge of treated wastewater will remain at its existing location, which is on the Ohio River at mile point 643.3, segment 08217. The average daily discharge rate is projected to increase from its current flow rate of approximately 0.232 MGD (million gallons per day) to 0.268 MGD by 2037. The existing WWTP, however, is already rated to treat 0.312 MGD.

We would appreciate your advice of any concerns your office may have related to possible adverse effects of these projects as soon as possible. We need to incorporate your response in the Facilities Plan, and also address your concerns regarding any potential adverse impacts of these projects.

It is anticipated that these projects will be funded by a series of grants and loans, such as the USDA Rural Development Agency Grant and Loan Program and the Kentucky Infrastructure Agency State Revolving Fund Loan Program.

Please feel free to contact me if you have any questions or comments. I may be reached at 502-489-8484 or at ngunselman@grwinc.com

Sincerely,

Nicholas Gunselman, P.E.

Project Engineer

Ms. Ruth S. Pike
Acting Supervisory Natural Resource Manager
USDA Natural Resources Conservation Services
Hardinsburg Service Center
1101 S. Highway 261
Hardinsburg, KY 40143

Cross-Cutter Correspondence Wastewater Facilities Plan Planning Period 2017-2037 City of Brandenburg, Kentucky GRW Project No. 4556

Dear Ms. Pike:

On behalf of the City of Brandenburg, GRW is preparing a Wastewater Facilities plan pursuant to the regulations of the Kentucky Division of Water, as required by 401 KAR 5:006. The purpose of this plan is to develop recommendations for a series of projects that will be constructed to improve wastewater collection and treatment in the City's Planning Area of the Planning Period of 2017 to 2037. Exhibit 2-6 illustrates the Planning Area Boundary and planning phases. Exhibit 5-1 illustrates the existing wastewater collection system.

Re:

The Facilities Plan includes a series of projects that will be completed over the next 20 years in phases. Some of these projects will be built within the wastewater collection system or at the treatment plant site.

The collection system projects include new gravity sewers, new lift stations and force mains, constructed in easements or along existing road right-of-way in order to sewer existing neighborhoods (see Exhibit 1-4). These collection system projects are expected to be completed within the 3-10 and 11-20 year planning phases. The City may or may not choose to construct the collection system projects.

The treatment plant project (see Exhibit 6-3.1), expected to be completed by December 31, 2021, includes concrete treatment structures, buried pipes, rehabilitation of existing structures, and the abandonment and/or demolition of some existing plant components that are no longer required for treatment. The treatment plant project will be on the existing plant site that was disturbed by the original construction of the plant in 1993. The discharge of treated wastewater will remain at its existing location, which is on the Ohio River at mile point 643.3, segment 08217. The average daily discharge rate is projected to increase from its current flow rate of approximately 0.232 MGD (million gallons per day) to 0.268 MGD by 2037. The existing WWTP, however, is already rated to treat 0.312 MGD.

We would appreciate your advice of any concerns your office may have related to possible adverse effects of these projects as soon as possible. We need to incorporate your response in the Facilities Plan, and also address your concerns regarding any potential adverse impacts of these projects.

It is anticipated that these projects will be funded by a series of grants and loans, such as the USDA Rural Development Agency Grant and Loan Program and the Kentucky Infrastructure Agency State Revolving Fund Loan Program.

Please feel free to contact me if you have any questions or comments. I may be reached at 502-489-8484 or at ngunselman@grwinc.com

Sincerely,

Nicholas Gunselman, P.E.

Project Engineer

# **GRW** | engineering | architecture | geospatial

9710 Bunsen Parkway | Louisville, KY 40299 502.489.8484 | www.grwinc.com

September 22, 2017

Ms. Pam J. Loeffler Regulatory Specialist U.S. Army Corps of Engineers Louisville District P.O. Box 59 Louisville, KY 40201 Re: Cross-Cutter Correspondence Wastewater Facilities Plan Planning Period 2017-2037 City of Brandenburg, Kentucky GRW Project No. 4556

Dear Ms. Loeffler:

On behalf of the City of Brandenburg, GRW is preparing a Wastewater Facilities plan pursuant to the regulations of the Kentucky Division of Water, as required by 401 KAR 5:006. The purpose of this plan is to develop recommendations for a series of projects that will be constructed to improve wastewater collection and treatment in the City's Planning Area of the Planning Period of 2017 to 2037. Exhibit 2-6 illustrates the Planning Area Boundary and planning phases. Exhibit 5-1 illustrates the existing wastewater collection system.

The Facilities Plan includes a series of projects that will be completed over the next 20 years in phases. Some of these projects will be built within the wastewater collection system or at the treatment plant site.

The collection system projects include new gravity sewers, new lift stations and force mains, constructed in easements or along existing road right-of-way in order to sewer existing neighborhoods (see Exhibit 1-4). These collection system projects are expected to be completed within the 3-10 and 11-20 year planning phases. The City may or may not choose to construct the collection system projects.

The treatment plant project (see Exhibit 6-3.1), expected to be completed by December 31, 2021, includes concrete treatment structures, buried pipes, rehabilitation of existing structures, and the abandonment and/or demolition of some existing plant components that are no longer required for treatment. The treatment plant project will be on the existing plant site that was disturbed by the original construction of the plant in 1993. The discharge of treated wastewater will remain at its existing location, which is on the Ohio River at mile point 643.3, segment 08217. The average daily discharge rate is projected to increase from its current flow rate of approximately 0.232 MGD (million gallons per day) to 0.268 MGD by 2037. The existing WWTP, however, is already rated to treat 0.312 MGD.

We would appreciate your advice of any concerns your office may have related to possible adverse effects of these projects as soon as possible. We need to incorporate your response in the Facilities Plan, and also address your concerns regarding any potential adverse impacts of these projects.

It is anticipated that these projects will be funded by a series of grants and loans, such as the USDA Rural Development Agency Grant and Loan Program and the Kentucky Infrastructure Agency State Revolving Fund Loan Program.

Please feel free to contact me if you have any questions or comments. I may be reached at 502-489-8484 or at ngunselman@grwinc.com

Sincerely,

Nicholas Gunselman, P.E.

Project Engineer

# **Appendix I**

**City and County Resolutions** 

# Appendix J

Public Meeting Minutes,

Public Meeting Presentation,

Attendance Roster,

Public Comments,

Affidavit of Publication,

and The Meade County

Messenger & Times Tear Sheets

# **Appendix K**

**Design Calculations** 

# I. OVERVIEW

A Lemna Environmental Technologies, Inc. Biological Treatment Process (BTP) can include various unit processes to achieve project-specific treatment goals. Lagoon unit processes include Complete Mix (CM), followed by Settling Cell. The Lemna Polishing Reactor (LPR) is a post-lagoon reactor designed for polishing lagoon effluent. The performance of each unit process is modeled using calculations formulated from published references and supported by field experience. The purpose of this document is to provide a summary of the theoretical basis for each calculation.

# II. ORGANICS REMOVAL

# A. Complete Mix - Mechanistic Approach

The removal of organics in the CM cell is calculated using a "mechanistic" model that relates the growth of heterotrophic bacteria to the removal of carbonaceous biochemical oxygen demand (CBOD<sub>5</sub>). The equations for determination of effluent concentration of CBOD<sub>5</sub> for a complete mix cell are provided below (Rich, 1999):

$$S_{e} = \frac{K_{s}(1 + k_{d} \times DT)}{DT(\mu_{m} - k_{d}) - 1}$$

Where:

S<sub>e</sub> = Effluent CBOD<sub>5</sub>, mg/L

k<sub>d</sub> = Bacterial Decay Rate, d<sup>-1</sup>

DT = Detention Time, d

K<sub>s</sub> = Half-Saturation Constant = 120 mg/L (default value),

μ<sub>m</sub> = Maximum Heterotrophic Specific Growth Rate, d<sup>-1</sup>

Maximum heterotrophic growth rate is adjusted for temperature using the following equations:

$$\mu_{\text{m}} = \mu_{\text{m20}} \times 1.026^{\text{T}-\text{20}}$$
 (for T between 10°C and 30°C)

$$\mu_{m} = 0.774 \times \mu_{m20} \times 1.058^{\text{T}-20}$$
 (for T between 2°C and 10°C)

Where:

 $\mu_{m20} = 6.0 \text{ d}^{-1}$  (Grady and Daigger, 1999)

T = Wastewater Temperature, °C

The following equations are used to calculate bacterial decay rate:

$$k_{\text{d20}} = 0.48 \times DT^{-0.415}$$

$$k_d = k_{d20} \times 1.05^{T-20}$$

For summer, complete mix cell 1A, T=20 ° C

$$\mu_{\rm m} = 6.0 \times 1.026^{20-20} = 6.0 \, d^{\text{-1}}$$

$$k_{d20} = 0.48 \times 3.0^{-0.415} = 0.30 \, d^{\text{-}1}$$

$$k_d = 0.30 \times 1.05^{20-20} = 0.30 \, d^{\text{-}1}$$

BOD summer effluent from the complete mix cell, 1A

$$S_e = \frac{120(1+0.30\times3.0)}{3.0(6.0-0.30)-1} = 14 \text{ mg/L}$$

For winter, complete mix cell 1A, T=8.2 ° C

$$\mu_m = 0.774 \times 6.0 \times 1.058^{8.2-20} = 4.2 \, d^{\text{-}1}$$

$$k_{d20} = 0.48 \times 3.0^{-0.415} = 0.30 \, d^{-1}$$

$$k_d = 0.30 \times 1.05^{8.2-20} = 0.17 \, d^{-1}$$

BOD winter effluent from the complete mix cell, 1A

$$S_e = \frac{120(1+0.17\times3.0)}{3.0(4.2-0.17)-1} = 16 \text{ mg/L}$$

# III. AMMONIA REMOVAL

# A. Complete Mix - Nitrification

The removal of ammonia in the CM cell in the summer that can be attributed to nitrifying bacteria is calculated using an equation that relates the removal of ammonia to the bacterial growth rate. The equations are provided below (Rich, 1999):

$$N = \frac{K_{N}}{DT \times \left(\mu_{m} \frac{O_{2}}{K_{O_{2}} + O_{2}} \left[1 - 0.83(7.2 - pH)\right]\right) - 1}$$

Where:

N = Effluent Ammonia, mg/L

 $\mu_m$  = Maximum Nitrifier Specific Growth Rate, d<sup>-1</sup>

O<sub>2</sub> = Dissolved Oxygen, mg/L

pH = Hydrogen Ion Concentration

K<sub>N</sub> = Half-Saturation Constant for Ammonium Nitrogen, mg/L

K<sub>O2</sub> = Half-Saturation Constant for Dissolved Oxygen = 1.2 mg/L

DT = Detention Time, d

 $K_{N}$  and  $\mu_{m}$  are adjusted for temperature using the following equation:

$$\mu_{\text{m}} = 10^{^{0.0413T-0.944}}$$

and

$$K_N = 10^{0.051T-1.158}$$

Where:

T = Wastewater Temperature, °C

For summer complete mix cell 1A, T=20 °C

$$\mu_{m} = 10^{0.0413\times20-0.944} = 0.767\,d^{-1}$$

$$K_N = 10^{0.051 \times 20 - 1.158} = 0.728 \, d^{-1}$$

NH<sub>3</sub> summer effluent from the complete mix cell 1 A

$$N = \frac{0.728}{3.0 \times \left(0.767 \frac{2}{1.2 + 2} \left[1 - 0.83(7.2 - 7.2)\right]\right) - 1} = 1.7 \text{ mg/L}$$

# B. Complete Mix - Heterotrophic Uptake

Removal of ammonia in CM cell in winter is partially accounted for by the uptake of nitrogen by heterotrophic bacteria to meet stoichiometric demands for cell synthesis. The ammonia used for synthesis of heterotrophic organisms is calculated using following equation (Grady and Daigger, 1999):

$$N_R = \Delta CBOD_5 \times Y \times 0.087$$

Where:

N<sub>R</sub> = Heterotrophic Nitrogen Requirements, mg/L

 $\Delta CBOD_5 = CBOD_5$  Removed, mg/L

Y = Bacterial Yield, mg biomass-COD/mg CBOD

0.087 = Nitrogen Content of Biomass, mg-N/mg-biomass-COD

$$N_R = 385 \times 0.5 \times 0.087 = 16.7 \,\text{mg/L}$$

NH<sub>3</sub> winter effluent from the complete mix cell 1 A

# C. Lemna Polishing Reactor - Nitrification

LPR media requirements for ammonia removal are calculated according to the following equation:

$$A = \frac{\Delta N H_3 \times Q \times 8.34}{G_T}$$

Where:

A = Media Surface Area, ft<sup>2</sup>

 $\Delta NH_3 = Ammonia Removed, mg/L$ 

Q = Flow, MGD

 $G_T = NH_3$  removal rate, lb  $NH_3/ft^2$ -media/day

LPR ammonia removal rate,  $G_{20}$  at 20 degrees Celsius, is 0.0003 lbs/ft<sup>2</sup>-media/day. The removal rate is adjusted for wastewater temperature using the following equation:

$$G_T = G_{20} (1.072)^{T-20}$$

Where:

$$G_{20}$$
 = reaction rate at 20°C  
T = design wastewater temperature, °C

For influent wastewater temperature of 3.2°C

$$G_T=0.0003(1.072)^{3.2-20}=0.00009$$
 lbs/ft<sup>2</sup>/day

For the influent ammonia concentration to the LPR of 27 mg-N/L assuming none ammonia removal in the lagoon;

$$A = \frac{(27-20) \times 0.312 \times 8.34}{0.00009} = 195890 \text{ ft}^2$$

For media cubes of 6' x 6' x 10' at 68 ft<sup>2</sup>/ft<sup>3</sup> density

NH<sub>3</sub> cubes required =8 cubes

Total modules provided =10 cubes

# IV. OXYGEN REQUIREMENT

# D. Complete Mix and LPR

The first step in the calculation of aeration requirements is the determination of the mass load (CBOD<sub>5</sub> and NH<sub>3</sub>) removed under worst-case conditions. The mass load, L, is calculated by multiplying the flow and concentration and correcting for units.

$$L_{CBOD5} = Q \times \Delta CBOD_5 \times 8.34$$
  
$$L_{NH_3} = Q \times \Delta NH_3 \times 8.34$$

Where:

$$\begin{split} &L_{CBOD5} = CBOD_5 \ Removed, \ lb-CBOD_5 \ /d \\ &L_{NH3} = NH_3 \ Removed, \ lb-NH_3 \ /d \\ &\Delta CBOD_5 = CBOD_5 \ Removed, \ mg-CBOD_5 \ /L \\ &\Delta NH_3 = NH_3 \ Removed, \ mg-NH_3 \ /L \\ &Q = Flow, \ MGD \end{split}$$

Cell #1A:  $8.34 \times 0.312 \times (401 - 14) \cong 1006 \text{ lb-CBOD}_5 / d$ 

 $8.34 \times 0.312 \times 27 \cong 70 \text{ lb-NH}_3 / d$ 

LPR:  $8.34 \times 0.312 \times (27-20) = 18 \text{ lb-NH}_3 / \text{ d}$ 

The next step is a calculation of the amount of "actual" oxygen required to support the removal of the calculated load. Factors of 1.0 lb  $O_2$ /lb  $CBOD_5$  and 4.6 lb  $O_2$ /lb  $NH_3$  are used to account for the bacterial oxygen demands in the CM cell (Note: oxygen demands for bacterial digestion are calculated separately).

$$\begin{aligned} & \text{AOR}_{\text{CBOD5}} = 1.0 \text{ x L}_{\text{CBOD5}} \\ & \text{AOR}_{\text{NH3}} = 4.6 \text{ X L}_{\text{NH}_3} \end{aligned}$$

Where:

AOR<sub>CBOD5</sub> = Actual Oxygen Requirements for CBOD<sub>5</sub> load, lb-O<sub>2</sub> /d

AOR<sub>NH3</sub> = Actual Oxygen Requirements for NH<sub>3</sub> load, lb-O<sub>2</sub> /d

1.0 = lb-O<sub>2</sub> required / lb CBOD<sub>5</sub> removed

 $4.6 = \text{lb-O}_2 \text{ required / lb NH}_3 \text{ removed}$ 

Cell #1A: 1,006 lb CBOD<sub>5</sub>/d x 1.0 lb-O<sub>2</sub>/lb CBOD<sub>5</sub> + 70 lb NH<sub>3</sub>/d x 4.6 lb-O<sub>2</sub>/lb NH<sub>3</sub> =

1328 lb-O<sub>2</sub>/d

LPR:  $18 \text{ lb NH}_3/\text{d x } 4.6 \text{ lb-O}_2/\text{lb NH}_3 = 83.8 \text{ lb-O}_2/\text{d}$ 

The actual oxygen requirements are adjusted to standard conditions using a correction factor. The correction factor accounts for various factors that will affect oxygen transfer from site to site.

$$SOR = \frac{AOR}{CF}$$

Where:

SOR = Standardized Oxygen Requirement, lb-O<sub>2</sub> /d

AOR = Actual Oxygen Requirement, lb-O<sub>2</sub> / d

CF = Correction Factor

The correction factor has the following equation:

$$CF = \alpha \times \left(\frac{\left(\beta \times C_s \times \delta\right) - RO}{C_{s20}}\right) \times F \times \Theta_{MT}^{T-20}$$

Summer:

$$CF = 0.75 \times \left(\frac{\left(0.95 \times 8.9 \times 17.0/14.4\right) - 2.0}{9.09}\right) \times 0.9 \times 1.024^{20-20} = 0.59$$

Winter:

$$CF = 0.75 \times \left(\frac{\left(0.95 \times 11.5 \times 17.0 / 14.4\right) - 2.0}{9.09}\right) \times 0.9 \times 1.024^{3.2-20} = 0.54$$

Where:

 $\alpha$  = Surface Tension Correction Factor = 0.75

 $\beta$  = Solubility Correction Factor = 0.95

C<sub>S20</sub> = O<sub>2</sub> Saturation at Standard Conditions = 9.09 mg/L

C<sub>S</sub> = Oxygen Saturation at Water Surface, mg / L

RO = Residual Oxygen = 2.0 mg / L

 $\Theta_{MT}$  = Temperature – Mass Transfer Correction Factor = 1.024

T = Design Water Temperature, °C

F= Diffuser Factor = 0.9

 $\delta$  = P<sub>eff</sub> / P = Pressure Correction Factor

P<sub>eff</sub> = Effective Pressure of Aeration, psia

$$P_{\text{eff}} = P + \left(0.433 \times \frac{\text{WD}}{2}\right)$$

$$P_{\text{eff}} = 14.4 \text{ psi} + (0.433 \text{ x } 12/2) = 17.0 \text{psi}$$

Where:

P = Site Barometric Pressure, psia

WD = Water Depth, ft

0.433 = Conversion Factor, feet to psia

Cell #1A: SOR =  $1328/0.59 \approx 2251 \text{ lb/d}$ 

LPR: SOR =  $83.8/0.54 \cong 155 \text{ lb/d}$ 

The SOR is used in combination with manufacturer supplied aeration efficiencies to compute the required air flow to meet the bacterial oxygen demand. The following equation is used for calculating air flow requirement:

$$Q_{air} = f \times \frac{SOR}{SOTE}$$

Where:

 $Q_{air}$  = Air Requirement, SCFM SOTE = Manufacturer's Transfer Efficiency, % f = Conversion Factor from lbs-O<sub>2</sub> /day to SCFM

Cell #1A: 
$$Q_{air} = 4.025 \times \frac{2251 \text{lb-} O_2 / d}{17.3 \%} = 523 \text{ SCFM}$$

LPR: 
$$Q_{air} = 4.025 \times \frac{155 \, lb \cdot O_2 \, /d}{16.1\%} = 38 \, SCFM$$

# E. Digestion

Ten State Standards requires 1.5 pounds of oxygen per pound of  $CBOD_5$  removed for aerated lagoons. The 0.5 portion of that factor is required for digestion (benthal stabilization). The sludge, in settling cell, is deposited on the lagoon floor. Over time, the sludge accumulates, compacts, and slowly digests. The amount of oxygen required to support benthal stabilization is calculated by the following equation:

$$AOR_D = 0.5 x L_{CBOD5}$$

Where:

 $L_{CBOD5} = CBOD_5$  removed, lb-CBOD<sub>5</sub> /d

AOR<sub>D</sub> = Actual Oxygen Requirement for Bacterial Digestion, lbs O<sub>2</sub>/d

 $AOR = 1,006 \text{ lb } BOD/d \times 0.5 \text{ lb-O}_2/\text{lb } BOD_5 = 503 \text{ lb } O_2/d$ 

$$SOR = 503/0.59 \cong 852 \text{ lb/d}$$

Cell #1B: 
$$Q_{air} = 4.025 \times \frac{852 \text{ lb} - O_2 / d}{17.3\%} = 198 \text{ SCFM}$$

90% of air flow required for digestion will be supplied in settling cell.

# V. Mixing Requirements

# F. Blower mixing

For the purpose of sizing aeration to achieve complete mix conditions, a dissipated energy of 13.5 HP/MG is used. This value is multiplied by the complete mix cell volume to determine the power dissipated, P.

$$P_{CM} = 13.5 \text{ HP/MG x Q x DT}$$

Where:

P<sub>CM</sub> = Dissipated Power, HP

Q = Flow, MGD

DT = Detention Time, d

 $P_{CM} = 13.5 \text{ HP/MG x } 0.312 \text{ MGD x } 3.0 \text{ d} = 12.6 \text{ HP} = 6930 \text{ ft-lb/s}$ 

Given a desired amount of energy dissipation into a volume of water, the following equation is used to determine an equivalent air flow (Metcalf & Eddy, 1991).

$$Q_a = \frac{P_{\text{CM}}}{K \times In \left(\frac{h + 33.9}{33.9}\right)}$$

Where:

Q<sub>a</sub> = Air Flow Rate, CFM

P<sub>CM</sub> = Power Dissipated, ft-lb/s

K = Constant (35.28, U.S. customary units)

h = Air Pressure at Discharge, ft (water)

Cell #1A: 
$$Q_a = \frac{6930}{35.28 \times In \left(\frac{12 + 33.9}{33.9}\right)} = 650 \, CFM$$

# **VI. Equipment Selection**

Equipment selection is based on aeration and/or mixing, whichever requires the highest horsepower.

Cell #1A: 38 diffusers x 18 SCFM = 684 SCFM

Cell #1B: 20 diffusers x 9 SCFM = 180 SCFM

LPR: 90 SCFM

Total air requirement equals:

The blower power motor requirement is calculated using the following equation:

$$BHP = \frac{Q_{air} \times f_2}{E_{blow}} \times \left( \left( \frac{P + P_{blow}}{P} \right)^{0.283} - 1 \right) \times h$$

Where:

BHP = Blower power motor requirement (HP, horsepower)

Conversion factor for air volume to air mass

 $f_2 = E_{blow} = P_{blow} =$ Blower efficiency (%) Pressure at Blower outlet Air humidity correction factor

954 SCFM

$$P_{blow} = 62.4/144 \text{ x (WD +3.0)} = 62.4/144 \text{ x (12+3.0)} = 6.50 \text{ psig}$$
  
 $E_{blow} = 78.7 - (2.11 \text{ x } P_{blow}) = 78.7 - (2.11 \text{ x } 6.50) = 65.0 \%$ 

BHP = 
$$(((954 \times 0.068)/60) \times 53.3 \times 595) / (550 \times 0.283 \times 0.650)) \times (((6.5 + 14.4)/14.4)^{0.283} - 1) \times 1.12$$
  
= 46.6 BHP

Suggested Blower Size:3 @ 30 HP (2 operating, 1 stand by)

# VII. Solids Accumulation

Bacteria and other solids accumulate in areas of the lagoon in which there is insufficient energy to keep the solids in suspension. Accumulated solids will undergo a process of cumulative digestion and compaction over time. The resulting biomass/sludge is largely comprised of inert solids. For the purpose of design, an influent inert solids concentration is utilized to calculate desludging volumes. The following equation provides an estimate of daily solids accumulation (adapted from Rich, 1999):

$$R = \frac{Q \times X_i \times 8.34}{W_s \times \rho}$$

Where:

R = Daily Accumulation Rate, cf/d

Q = Flow, MGD

X<sub>i</sub> = Inert Solids Concentration, mg/L = 169 mg/L

W<sub>s</sub> = Weight Fraction of Solids = 5%

 $\rho$  = Density of Water = 62.4 lbs/cf

The volume of deposited sludge in a year:

$$R_y = \frac{0.312 \times 169 \times 8.34 \times 7.48 \times 365}{62.4 \times 0.05 \times 1000000} = 0.38 \, \text{MG/year}$$

Assuming that desludging will be performed at half of the settling cell volume filled

$$Des ludging Interval = \frac{V_{settling\_eell} \times \frac{1}{2}}{V_{sludge}} = \frac{2.56 \, MG \times \frac{1}{2}}{0.38 \, MG/year} = 3.3 \, years$$

# VIII. References

Abd-El-Bary, M.F., Eways, M.J., and Mast Engineering Co., Inc.: "Biological Nitrification in Contact Aeration Systems", Water & Sewage Works, June 1977.

Grady, C.P.L. Jr., G.T. Daigger and H.C. Lim: *Biological Wastewater Treatment*, Marcel-Dekker, Inc., New Your, 1999.

Great Lakes-Upper Mississippi River Board of State and Provincial Public Health and Environmental Managers: Recommended Standards for Wastewater Facilities (Ten State Standards), 1997.

Metcalf & Eddy, Inc.: Wastewater Engineering: Treatment, Disposal, and Reuse, McGraw-Hill, Inc., New York, 1991.

Rich, L.G.: *High Performance Aerated Lagoons*, American Academy of Environmental Engineers, 1999.

U.S. Environmental Protection Agency: *Design Manual – Fine Pore Aeration Systems*, EPA/625/1-89/023, Cincinnati, OH, September 1989.

U.S. Environmental Protection Agency: *Design Manual – Municipal Wastewater Stabilization Ponds*, EPA/625/1-83-015, Cincinnati, OH, September 1983.

U.S. Environmental Protection Agency: *Design Manual – Nitrogen Control*, EPA/625/R-93/010, Cincinnati, OH, October 1983.

Water Environment Federation: *Design of Municipal Wastewater Treatment Plants*, Manual of Practice No. 8, 1992.

Water Environment Federation: *Natural Systems for Wastewater Treatment*, Manual of Practice FD-16, 1990.

Water Environment Federation: Aeration, Manual of Practice - MOP FD-13, 2001.

Lemna En	vironmenta	l Technolog	gies, Inc.	į	Engineerin	g				Brandenb	urg, KY 8/1/2017
Wastewater I Flow BOD TSS Ammonia Total Nitrogen Phosphorus		Summer 0.312 401 371 27	Winter 0.312 401 371 27	MGD mg/L mg/L mg/L	30 30 30 20	Winter  30 30 20 -	mg/L mg/L mg/L mg/L mg/L	Site Data Winter Air Ter Winter Air Ter Elevation Atmospheric F Distance to Si	nperature Pressure	32 0.0 570 14.4 750	"F "C ft AMSL psia miles
Lagoon Desig Basin # 1 Included? Influent Tempi Flow Covered? Water Depth Freeboard Slope Length (bottor Width (waterli Length (waterli Length (pottor Width (bottor Width (bottor Width (bottor Width (bottor Vidth (bottor Length (at top Width (at top Width (at top Volume Volume Detention Tim Selected R (N Covered Basin Uncovered Basin Uncovered Basin	erature  iine) he) n) of berm) e e mominal)	Yes 10.0 312 No 112.0 2.0 3.0 3.25 170 253 98 337 182 55,250 24,794 19.2 8.0 9.3 3.5 11.2 8.0 3.5 11.2 8.0 3.3 3.2	"C MGD ft to 1 to 1 ft f	ı							
Cell Sizing Cell 1A 1B	Mixing CM SC	Det Time (d) 3.0 8.2	Depth (ft) 12.0 12.0	Winter Temp. 8.2 3.2	Rate (d <sup>-1</sup> ) 4.2	CBOD <sub>5</sub> In 401 16	CBOD <sub>5</sub> Out 16 16	NH <sub>3</sub> In 27.0 10.3	NH <sub>3</sub> Out 10.3 10.3	Nitrification? no no	
Cell 1A 1B	Mixing CM SC	Det Time (d) 3.0 8.2	Depth (ft) 12.0 12.0	20.0 20.0	Rate (d") 6.0	CBOD <sub>S</sub> In 401 14	CBOD <sub>5</sub> Out 14 14	NH <sub>3</sub> In 27.0 1.7	NH <sub>3</sub> Out 1.7 1.7	Nitrification? yes no	
Cell 1A 1B	Mixing Requir  Mixing  CM  SC	CBOD <sub>5</sub> (lb/d) 1,006	NH <sub>3</sub> (lb/d) 70 0	CBOD <sub>5</sub> (SCFM) 396	NH <sub>3</sub> (SCFM) 127 0	Mixing (SCFM) 650	Benthal Air (SCFM) 20 178	Asp. Air (HP) 55 19	Sup. Mixer (HP) -1	Nitrification Air? Yes No	Benthal Air (%) 10% 90%
Equipment Se	election	Air per									1
Cell 1A 1B	Diffuser Type HR LR	Diffuser (SCFM) 18	No. of Diffusers 38 20	No. of Laterals 5 4	Lateral Length (ft) 134 134	No. of Units	Aspirator - Hp		Mixer - Hp	Air Flow (SCFM) 684 180 0	
Total  Stabilization Estimated Sta Biodegradable Nondegradabl Stabilzation Lo	Solids e Solids		40,484 345 439 42	sf lbs/d lbs/d g-solids/m²/d		Sludge Densit Rate of Sludge Desludging Vo	Accumulation olume	0	5% 0.38 1.28 3.3	MG/year MG years	J
LPR Design Cube Length Cube Length Cube Width Cube Depth Temperature Influent GBOC Effluent CBOC BOD Load BOO Loab BOO Loab BOO Lobe BO BOO Lobe Loading Rate BOO Cube Lo Loading Rate BOO Cube Lo Influent Ammo Effluent Ammo Ammo Rate Loading Rate NH3 Cube De Loading Rate NH3 Cube R	nsity equired ading Rate inia onia d nsity		Winter 6 6 10 3.2 16.4 16.4 0 48 0.00138 0.0 27.0 20.0 18 68 0.00009	ft ft deg. C mg/L mg/L lbs/d st/d b-CBODs/st/d Cubes lb-CBODs/kct/l mg/L bs/day st/cf lb-NH_s/st/d Cubes		LPR Aeration Minimum Cub Supplemental Total Cubes Water Depth BOD Oxygen NH, Oxygen R Total Oxygen Diffuser SOTE Transfer Effici LPR Aeration LPR Mixing Channels Spaces per Cl Unused Space Detention Tim	es Cubes  Requirement Requirement Requirement ency		Winter  8 2 10 12 0 83.8 83.8 83.8 83.8 93.9 1.4% 16.1% 38 90 2 5 0 3.9	Cubes Cubes Cubes th bs/day bs/day bs/day SCFM SCFM Channels Cubes Spaces Hours	
Blower Sizing Maximum War Aeration Req. Mass Air Flow Outlet Blower	ter Depth		954 1.2 6.50	Feet SCFM lb/s psig		Blower Efficier Blower Motor I Number of Blo Suggested Bi	Power Req. wers		65.0 46.6 3 30	% BHP Units HP	

# City of Brandenburg Wastewater Facilities Plan

# Aeration Requirements (Historic 2014-2016 Data)

# 9/22/2017

# **Design Data**

Wastewater	· Characteristics
------------	-------------------

Average Flow	0.232 MGD	(2014-2016)
Peak Flow	0.385 MGD	(2014-2016)

Wastewater Temp 22 °C (summer) (assumed Aqua-Aerobic) 12 °C (winter) (assumed Aqua-Aerobic)

 $\begin{array}{cccc} \text{Influent BOD} & 401 \text{ mg/I} & (2014-2016) \\ \text{Influent NH}_3\text{N} & 27 \text{ mg/I} & (\text{Design}) \end{array}$ 

Influent TKN 45 mg/l (Metcalf & Eddy conversion)

**Basin Dimensions** 

225 ft WS Length WS Width 225 ft **Bottom Length** 120 ft **Bottom Width** 120 ft 17.5 ft Water Depth Side Slope 3:01 4 MG Volume Material Lined

Elevation 465 ft (assumed Aqua-Aerobic)

#### **Calculations**

Hydraulic Retention Time

HRT 17.2 days

Actual Oxygen Requirement

Oxygen Demand 1.5 lb  $O_2$  / lb BOD

 $4.6 \text{ lb O}_2 / \text{lb TKN}$ 

AOR (BOD) 48.5 lb  $O_2$  / hr AOR (TKN) 16.7 lb  $O_2$  / hr AOR 65.2 lb  $O_2$  / hr

Field Oxygen Transfer Efficiency

FTE  $\frac{\text{SOTE X [(Cs x \beta) - Cr] x 1.024}^{(t-20)} \text{ x } \alpha}{9.09}$ 

where:

SOTE  $3 lb O_2 / BHP-hr$ 

T 22 °C

Cs 8.59 mg/l (at 22°C and 465 ft)

 $\beta$  0.95 (typical, assumed)  $\alpha$  0.85 (typical, assumed)

Cr 2 mg/l

FTE 1.81

Power Requirements (for BOD and TKN)

Power (aeration) 39.1 HP based on 24 hours of aeration

OK

# City of Brandenburg Wastewater Facilities Plan Clarifier Sizing Design Calculations 9/21/2017

Flow Conditions:  $Q_{Avg}$  0.312 MGD

Q<sub>Peak</sub> 0.9 MGD

Q<sub>Peak RAS</sub> 0.468 MGD 150% Q<sub>Avg</sub> Per 10 States

**Existing Clarifiers** 

No. 2 diameter 36 ft  $A_{\text{each}}$  1,018 ft<sup>2</sup>

Surface Areas Existing Clarifiers 2,036 ft<sup>2</sup>

# **Total Area Required Based on Surface Overflow Rate Calculation**

SA<sub>required</sub> =Q<sub>Peak</sub>/Surface Overflow Rate at Design Peak Hourly Flow

Surface Overflow Rate at Design Peak Hourly Flow 1,000 gpd/ft<sup>2</sup> Per 10 States

SA<sub>required</sub> 932 ft<sup>2</sup>

 $SA_{req} < SA_{ex}$  OK

# **Total Area Based on Solids Loading Rate Calculation**

 $Q_{in} = Q_{Peak} + Q_{Peak RAS}$   $Q_{in} = 1.400 MGD$ 

MLSS 300 mg/L Per Falmouth Design Manual (200-300 mg/l)

MLVSS 240 mg/L 80% of MLSS

SLR<sub>Max</sub> 35.0 lb/day/ft<sup>2</sup> Per 10 States

 $SLR = (Q_{Peak} + Q_{Peak RAS}) * MLSS / SA$ 

SLR = 1.7

 $SLR < SLR_{Max}$ ? OK